

research report

Web Crippling and Bending Interaction of Cold-Formed Steel Members

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for the Design of Cold-Formed
Steel Structural Members



American Iron and Steel Institute

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**WEB CRIPPLING AND BENDING INTERACTION
OF
COLD-FORMED STEEL MEMBERS**

FINAL REPORT

**AN
AISI SPONSORED PROJECT**

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ABSTRACT

A new web crippling design approach has recently been adopted by the American Iron and Steel Institute and the *North American Specification for the Design of Cold-Formed Steel Structural Members, November 9, 2001 Draft Edition* (NAS, 2001). The current web crippling and bending interaction equations for single-web sections in the Specification (NAS, 2001) are based on the previous web crippling methods. As well, the moment component of the interaction equations was based on the previous reduced web strength method instead of the stress gradient approach that is now contained in the Specification (NAS, 2001).

Based the available data found in the literature, regression analyses were carried out using the new web crippling equations and the stress gradient method to substantiate the current web crippling and bending interaction equations in the Specification (NAS, 2001).

Based on the results of this investigation, new web crippling and bending interaction equations have been developed.

ACKNOWLEDGEMENT

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INTRODUCTION

With the recent adoption of the new web crippling design approach, the web crippling and bending interaction equations contained in the *North American Specification for the Design of Cold-Formed Steel Structural Members, November 9, 2001 Draft Edition* (NAS 2001) need to be re-evaluated. The *Specification* contains interaction equations for single web geometry, I-beam geometry, and two nested Z-shapes.

Changes in the moment strength calculation (effective web method) were introduced since the interaction equations were initially developed (Hetrakul and Yu, 1987) and hence, may also influence the combined web crippling and bending evaluation. Because of the changes in the web crippling and the bending strength calculations, the validity of the interaction equations must be investigated.

Two different formats for applying the phi and omega factors for the web crippling and bending interaction equations currently exist in the Specification (NAS, 2001). One format uses multiple phi and omega values for web crippling and bending, respectively, whereas the other format uses only one phi and one omega value on the combined behaviour. The application of phi and omega needs to be clarified by the Committee on Specification, and therefore is not discussed herein.

SCOPE OF STUDY

The scope of this project was to review the available data and determine the validity of the present interaction equations for the geometric section types. The current interaction equations were evaluated using the appropriate available interaction web crippling data found in the literature for single-web sections, built-up sections and nested Z-sections. Multi-web deck sections were not considered in this study. Based on the results of this study, new web crippling and bending interaction equations are proposed for adoption by the AISI Committee on Specification for the Specification (NAS, 2001).

LITERATURE REVIEW

Web Crippling Only

Four different load categories for web crippling are addressed by the Specification (NAS, 2001). These categories, illustrated in Figure 1, are: Interior One Flange (IOF) loading, Interior Two Flange (ITF) loading, End One Flange (EOF) loading, and End Two Flange (ETF) loading. In the case of IOF loading there is generally an interaction of web crippling and bending possible. Currently in the Specification (NAS, 2001), if the moment ratio, M/M_n , is ≤ 0.3 for single web members and 0.4 for I-sections, the web crippling and bending interaction need not be considered.

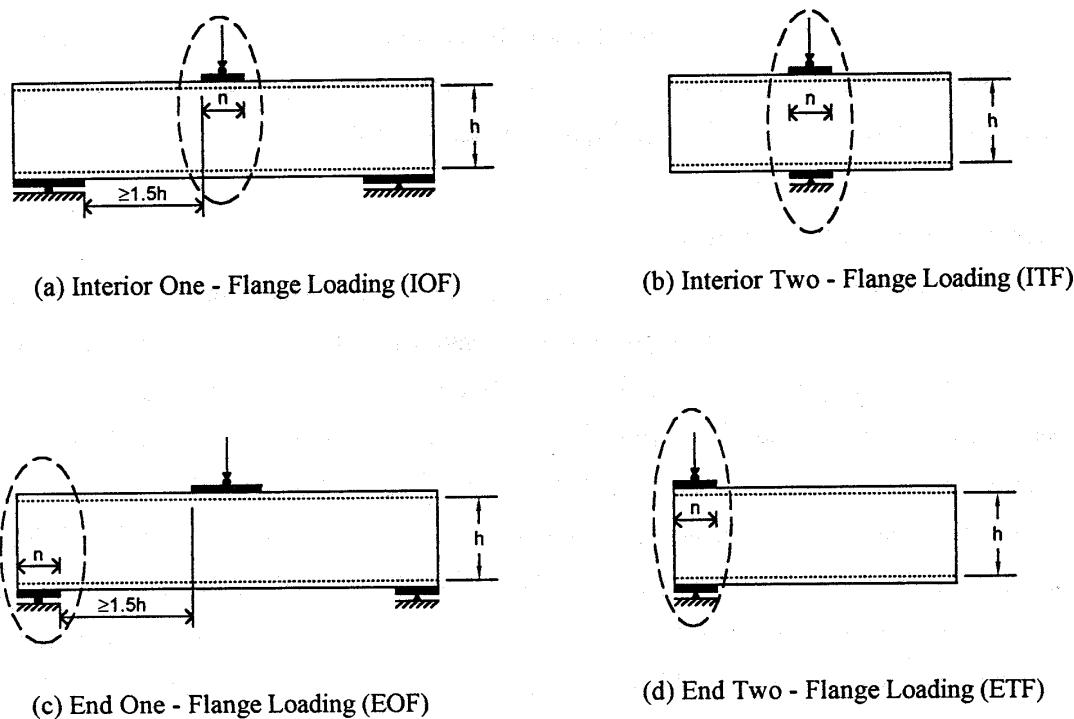


Figure 1: Load Categories for Web Crippling

The nominal web crippling strength of unreinforced webs is determined by the following equation:

$$P_n = C t^2 F_y \sin(\theta) \left\{ 1 - C_R \sqrt{\frac{R}{t}} \right\} \left\{ 1 + C_N \sqrt{\frac{N}{t}} \right\} \left\{ 1 - C_h \sqrt{\frac{h}{t}} \right\} \quad \text{Eq. C3.4.1-1 (NAS 2001)}$$

Where:

C = Overall web crippling coefficient

t = Web thickness

F_y = Yield stress of steel

θ = Angle between plane of web and plane of bearing surface $45^\circ \leq \theta \leq 90^\circ$

C_R = Inside bend radius coefficient

R = Inside bend radius

C_N = Inside bearing length coefficient

N = Bearing length [3/4 in. (19 mm) minimum]

C_h = Web slenderness coefficient

h = Flat dimension of web measured in plane of web

The web crippling coefficients C , C_R , C_N and C_h are summarized by Tables C3.4.1-1 to C3.4.1-4 which were taken from the Specification (NAS 2001).

**TABLE C3.4.1-1
BUILT-UP SECTIONS**

Support and Flange Conditions		Load Cases		C	C_R	C_N	C_h	Ω_w	ϕ_w	Limits
FASTENED TO SUPPORT	Stiffened or Partially Stiffened flanges	One - Flange Loading or Reaction	End	10	0.14	0.28	0.001	2.00	0.75	$R/t \leq 5$
			Interior	20	0.15	0.05	0.003	1.65	0.90	$R/t \leq 5$
UNFASTENED	Stiffened or Partially Stiffened Flanges	One - Flange Loading or Reaction	End	10	0.14	0.28	0.001	2.00	0.75	$R/t \leq 5$
			Interior	20.5	0.17	0.11	0.001	1.75	0.85	$R/t \leq 3$
	Unstiffened Flanges	Two - Flange Loading or Reaction	End	15.5	0.09	0.08	0.04	2.00	0.75	$R/t \leq 3$
			Interior	36	0.14	0.08	0.04	2.00	0.75	

Notes:

(1) This Table applies to I-beams made from two channels connected back to back.

See Section C3.4 of Commentary for explanation.

(2) The above coefficients apply when $h/t \leq 200$, $N/t \leq 210$, $N/h \leq 1.0$ and $\theta = 90^\circ$.

TABLE C3.4.1-4
SINGLE HAT SECTIONS

Support Conditions	Load Cases		C	C _R	C _N	C _h	Ω _w	ϕ _w	Limits
FASTENED TO SUPPORT	One - Flange Loading or Reaction	End	4	0.25	0.68	0.04	2.00	0.75	R/t ≤ 4
		Interior	17	0.13	0.13	0.04	1.90	0.80	R/t ≤ 10
	Two - Flange Loading or Reaction	End	9	0.10	0.07	0.03	1.75	0.85	R/t ≤ 10
		Interior	10	0.14	0.22	0.02	1.80	0.85	
UNFASTENED	One - Flange Loading or Reaction	End	4	0.25	0.68	0.04	2.00	0.75	R/t ≤ 4
		Interior	17	0.13	0.13	0.04	1.70	0.90	R/t ≤ 4

Note:

The above coefficients apply when h/t ≤ 200, N/t ≤ 200, N/h ≤ 2 and θ = 90°.

Bending Only

The nominal moment strength, M_{nxo}, of a cold-formed steel member is determined using the following expression:

$$M_{nxo} = S_e F_y, \quad \text{Eq. C3.1.1-1 (NAS 2001)}$$

where F_y is the yield stress of the material and S_e is the effective section modulus computed at a stress of f = F_y.

Combined Web Crippling and Bending

The interaction of web crippling and bending is evaluated by Eqs. 1, 2 and 3. These nominal equations serve as the basis for the design equations contained in the Specification (NAS, 2001).

For shapes having single unreinforced webs

$$1.07 \left(\frac{P_t}{P_n} \right) + \left(\frac{M_t}{M_n} \right) \leq 1.42 \quad \text{Eq. 1}$$

For I-sections such as two C-sections back-to-back

$$0.82 \left(\frac{P_t}{P_n} \right) + \left(\frac{M_t}{M_n} \right) \leq 1.32 \quad \text{Eq. 2}$$

For two nested Z-shapes

$$0.85 \left(\frac{P_t}{P_n} \right) + \left(\frac{M_t}{M_n} \right) \leq 1.65 \quad \text{Eq. 3}$$

REVIEW OF DATA

The focus was on three geometric shapes, or cross sections:

- (1) Shapes having single unreinforced webs
- (2) Built-Up Sections such as I-sections
- (3) Two nested Z-shapes

The cross-section type and dimensional notations for each test specimen type are shown in Figures A1 through A6 of Appendix A. Figure A1 shows the various I-sections and Figure A2 the types of hat sections tested by Winter and Pian (1946). Figure A3 shows the channel C-section and Figure A4 the stiffened and unstiffened I-sections tested by Hettrakul and Yu (1978). Figure A5 shows the nested Z-sections tested by LaBoube, Nunnery and Hodges (1994) and Figure A6 the unstiffened channel C-sections tested by Young and Hancock (2000).

The cross-sectional dimensions of the test specimens are summarized in Tables B1 through B7 of Appendix B. More specifically, Table B1 contains the data of the channel C-sections as shown in Figure A3, Table B2 the data of the hat sections as shown in Figure A2, Table B3 the data of the channel C-sections tested by Ratliff (1975), Table B4 the data of the unstiffened flange channel C-sections tested by Young and Hancock (2000), Table B5 the data of the stiffened flange I-sections as shown in Figure A4, Table B6 the data of the stiffened flange I-sections as shown in Figure A1 and Table B7 the data of the unstiffened flange I-sections as shown in Figure A4. No detailed dimensional data was given for the nested Z-section tests.

REVIEW AND DEVELOPMENT OF INTERACTION EQUATIONS

The accuracy of Equations 1, 2, and 3 was determined by using the ratio of the following relationships:

$$[A/1.42]; \text{ where } \mathbf{A} = 1.07 \left(\frac{P_t}{P_n} \right) + \left(\frac{M_t}{M_n} \right) \quad \text{Eq. 4}$$

$$[B/1.32]; \text{ where } \mathbf{B} = 0.82 \left(\frac{P_t}{P_n} \right) + \left(\frac{M_t}{M_n} \right) \quad \text{Eq. 5}$$

$$[C/1.65]; \text{ where } \mathbf{C} = 0.85 \left(\frac{P_t}{P_n} \right) + \left(\frac{M_t}{M_n} \right) \quad \text{Eq. 6}$$

Proposed Interaction Equations:

Based on a review of the test data, the following interaction equations were developed:

- (1) For shapes having single unreinforced webs

C-sections using the data from Hettrakul & Yu (1978), Ratliff (1975), and Young (2000),

$$0.89 \left(\frac{P_t}{P_n} \right) + \left(\frac{M_t}{M_n} \right) \leq 1.32 \quad \text{Eq. 7}$$

For C-sections and hat sections using the data from Hettrakul & Yu (1978),

Ratliff (1975), Young (2000), and Cornell (1953),

$$0.91 \left(\frac{P_t}{P_n} \right) + \left(\frac{M_t}{M_n} \right) \leq 1.33 \quad \text{Eq. 8}$$

- (2) For I-sections such as two C-sections back-to-back

Using the data from Hettrakul and Yu (1978),

$$0.88 \left(\frac{P_t}{P_n} \right) + \left(\frac{M_t}{M_n} \right) \leq 1.46 \quad \text{Eq. 9}$$

The Hettrakul & Yu (1978) data was based on double web I-sections
(see Figure A4 of Appendix A).

Using the data from Hettrakul & Yu (1978) and Winter & Pian (1946),

$$0.64 \left(\frac{P_t}{P_n} \right) + \left(\frac{M_t}{M_n} \right) \leq 1.29 \quad \text{Eq. 10}$$

The Winter & Pian (1946) data was based on single and double web I-sections
(see Figure A1 of Appendix A).

- (3) For two nested Z-shapes

Using the data from LaBoube, Nunnery and Hodges (1994)

$$0.86 \left(\frac{P_t}{P_n} \right) + \left(\frac{M_t}{M_n} \right) \leq 1.65 \quad \text{Eq. 11}$$

A graphical presentation of the above equations is shown in Appendix C.

The accuracy of Equations 7, 9 and 11 was determined by the following ratios:

$$[A'/1.32]; \text{ where } A' = 0.89 \left(\frac{P_t}{P_n} \right) + \left(\frac{M_t}{M_n} \right) \quad \text{Eq. 12}$$

$$[B'/1.46]; \text{ where } B' = 0.88 \left(\frac{P_t}{P_n} \right) + \left(\frac{M_t}{M_n} \right) \quad \text{Eq. 13}$$

$$[C'/1.65]; \text{ where } C' = 0.86 \left(\frac{P_t}{P_n} \right) + \left(\frac{M_t}{M_n} \right) \quad \text{Eq. 14}$$

The statistical accuracy of equations 8 and 10 was also determined by using ratios similar to Equations 12 and 13.

CALIBRATIONS

Resistance factors, ϕ , are used with the LRFD design method in the US and Mexico and with the LSD design method in Canada (NAS, 2001). The resistance factors are determined in conformance with each country's respective load factors and dead to live load ratios to provide a target reliability index, β , value of 2.5 for the US and Mexico and 3.0 for Canada (NAS, 2001). A satisfactory design can be obtained by equating the factored resistance to the factored loads, as follows:

$$\phi R_n = c(\alpha_D D_n + \alpha_L L_n) \quad \text{Eq. 15}$$

Where R_n is the nominal resistance and α_D and α_L are the dead and live load factors, respectively, such that the load combinations are 1.2D + 1.6L for the US and Mexico and 1.25D + 1.5L for Canada (NAS, 2001). The dead to live load ratios, D/L, are 1/5 for the US and Mexico and 1/3 for Canada (NAS, 2001).

Considering Equation 15, it can be shown that the resistance factors, ϕ , can be determined as follows.

$$\text{For AISI} \quad \phi = \frac{1.521(P_m M_m F_m)}{e^{\beta \sqrt{V_R^2 + V_Q^2}}} \quad \text{Eq. 16}$$

$$\text{For S136} \quad \phi = \frac{1.420(P_m M_m F_m)}{e^{\beta \sqrt{V_R^2 + V_Q^2}}} \quad \text{Eq. 17}$$

Where:

$$V_R = \sqrt{V_P^2 + V_M^2 + V_F^2} \quad \text{Eq. 18}$$

$$V_Q = \frac{\sqrt{(D_m V_D)^2 + (L_m V_L)^2}}{D_m + L_m} \quad \text{Eq. 19}$$

- P_m = mean ratio of experimental to calculated results
- M_m = mean ratio of actual yield point to minimum specified value
- F_m = mean ratio of actual to specified section modulus
- D_m = mean dead load intensity ($= 1.05 D_n^*$)
- L_m = mean live load intensity ($= L_n^*$)

* Values recommended by Hsiao et al. (1998)

D_n	= nominal dead load intensity
L_n	= nominal live load intensity
V_P	= Factor of variation of experimental to calculated results
V_M	= Factor of variation reflecting material properties' uncertainties
V_F	= Factor of variation reflecting geometric uncertainties
V_D	= Factor of variation of the dead load intensities
V_L	= Factor of variation of the live load intensities

The values of $M_m = 1.10$, $V_M = 0.08$, $F_m = 1.00$, and $V_F = 0.05$ were taken from Table F1 – Statistical Data for the Determination of Resistance Factor in (NAS, 2001). By knowing the resistance factor, ϕ , the corresponding factor of safety, Ω , can be computed as follows:

$$\text{For the US and Mexico} \quad \Omega = \frac{1.2 D/L + 1.6}{\phi(D/L + 1)} = 1.533/\phi \quad \text{Eq. 20}$$

Summarised in Tables 1 to 3 are the statistical values and corresponding factors of safety, Ω , and resistance factors, ϕ , calculated for the given test data.

DISCUSSION OF RESULTS

Experimental and calculated results are summarized in Tables B8 through B13 of Appendix B and the respective interaction diagrams are shown in Figures C1 through C5 of Appendix C. The statistical data for the interaction equations are summarized in the Tables 1 to 3. More specifically, shown in Table 1 are the results of single-unreinforced web sections, in Table 2 the results of I-sections such as two channel C-sections back to back and in Table 3 the results of nested Z-sections. The statistical results in these tables are self-explanatory.

Table 1 - Shapes having Single-Unreinforced Webs

Equation	Data Source	Mean	COV	U.S. & Mexico		ϕ
				Ω	ϕ	
Eq. 1 (Current)	Hetrakul (1978) Ratliff (1975) Young (2000)	1.025	0.097	1.69	0.907	0.782
Eq. 1 (Current)	Hetrakul (1978) Ratliff (1975) Young (2000) Cornell (1953)	1.021	0.097	1.70	0.903	0.779
Eq. 7 (Proposed)	Hetrakul (1978) Ratliff (1975) Young (2000)	1.004	0.101	1.73	0.885	0.762
Eq. 8 (Proposed)	Hetrakul (1978) Ratliff (1975) Young (2000) Cornell (1953)	0.999	0.101	1.74	0.880	0.759

Table 2 - Stiffened Flange I-Sections

Equation	Data Source	Mean	COV	U.S. & Mexico		Canada ϕ
				Ω	ϕ	
Eq. 2 (Current)	Hetrakul (1978)	1.071	0.094	1.61	0.950	0.820
Eq. 2 (Current)	Hetrakul (1978) Winter (1946)	1.097	0.104	1.59	0.964	0.830
Eq. 9 (Proposed)	Hetrakul (1978)	1.001	0.091	1.72	0.891	0.769
Eq. 10 (Proposed)	Hetrakul (1978) Winter (1946)	0.997	0.110	1.76	0.871	0.748

Table 3 - Nested Z-Sections

Equation	Data Source	Mean	COV	U.S. & Mexico		Canada ϕ
				Ω	ϕ	
Eq. 3 (Current)	LaBoube (1994)	0.991	0.034	1.68	0.914	0.798
Eq. 11 (Proposed)	LaBoube (1994)	0.998	0.033	1.66	0.921	0.804

In the case of I-sections such as two channel C-sections, the Hetrakul & Yu (1978) data was considered separately because the specimens were all double web I-sections made up of two channel C-sections, resulting in Eq. 9. The data from Winter & Pian (1946) on the other hand was made up of single and double web specimens. By considering both data sets, Eq. 10 was developed. By comparing Eq. 9 with Eq. 10, one can conclude that there is a considerable difference in interaction coefficients. Since most of the I-sections used in practice today are double web sections, it would be logical to use Eq. 9 instead of Eq. 10.

Figures C1 to C5 show that the new interaction equations yield an increase in computed strength and the statistical data in Tables 1 to 3 indicates that the new interaction equations provide an improved assessment of the interaction of web crippling and bending capacity when compared to the current interaction equations. As can be concluded from Table B13, insufficient data exists to draw final conclusions for I-sections having unstiffened flanges.

CONCLUSIONS AND RECOMMENDATIONS

The current web crippling and bending interaction equations in the Specification (NAS, 2001) have been evaluated using the available data found in the literature. The objective of this study was to evaluate the current interaction equations in light of the recent changes in both the pure web crippling equations and the bending strength determination.

Based on this study, new web crippling and bending interaction equations have been developed. In each case, the typical statistical parameters have been established to substantiate the best data-fit interaction equations. Shown in Table 4 are the recommended interaction equations with their respective factors of safety and resistance factors rounded off to the nearest 0.05.

Table 4 – Recommended Interaction Equations

Equation	Mean	COV	U.S. & Mexico		Canada ϕ
			Ω	ϕ	
C- and Hat Sections $0.91\left(\frac{P_t}{P_n}\right) + \left(\frac{M_t}{M_n}\right) \leq 1.33$	0.999	0.101	1.70	0.90	0.75
I-Sections $0.88\left(\frac{P_t}{P_n}\right) + \left(\frac{M_t}{M_n}\right) \leq 1.46$	1.001	0.091	1.70	0.90	0.75
Nested Z-Sections $0.86\left(\frac{P_t}{P_n}\right) + \left(\frac{M_t}{M_n}\right) \leq 1.65$	0.998	0.033	1.70	0.90	0.80

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APPENDIX A - GEOMETRIC SECTIONS INVESTIGATED

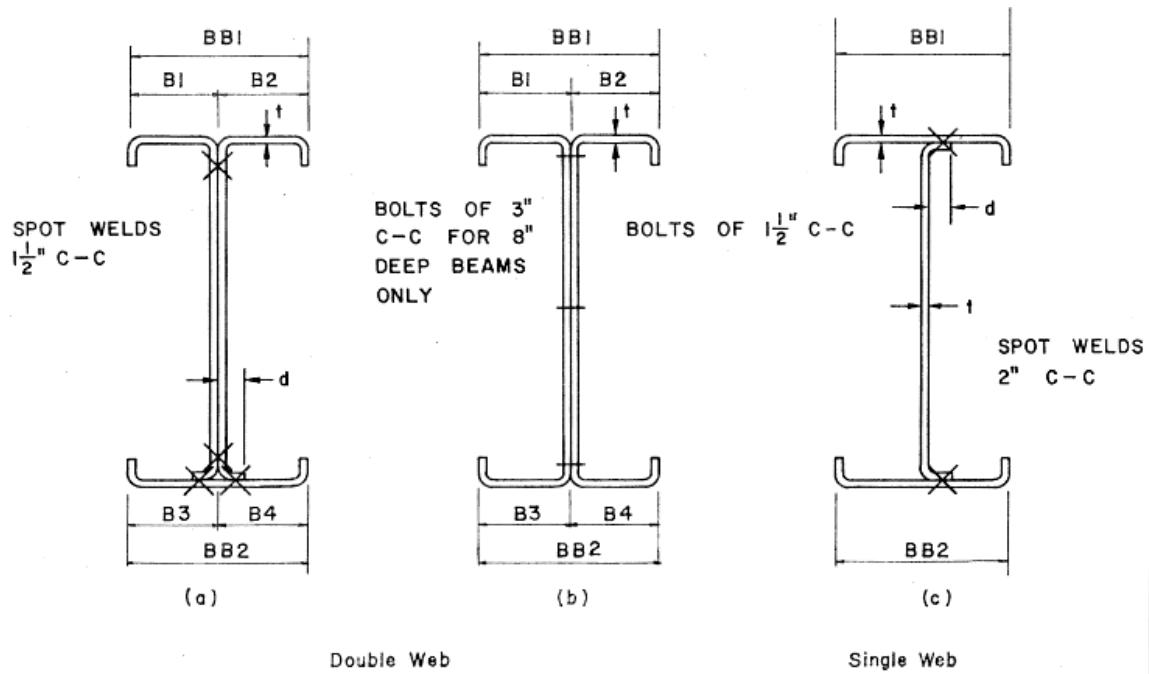


Figure A1: I-sections Tested by Winter and Pian (1946)

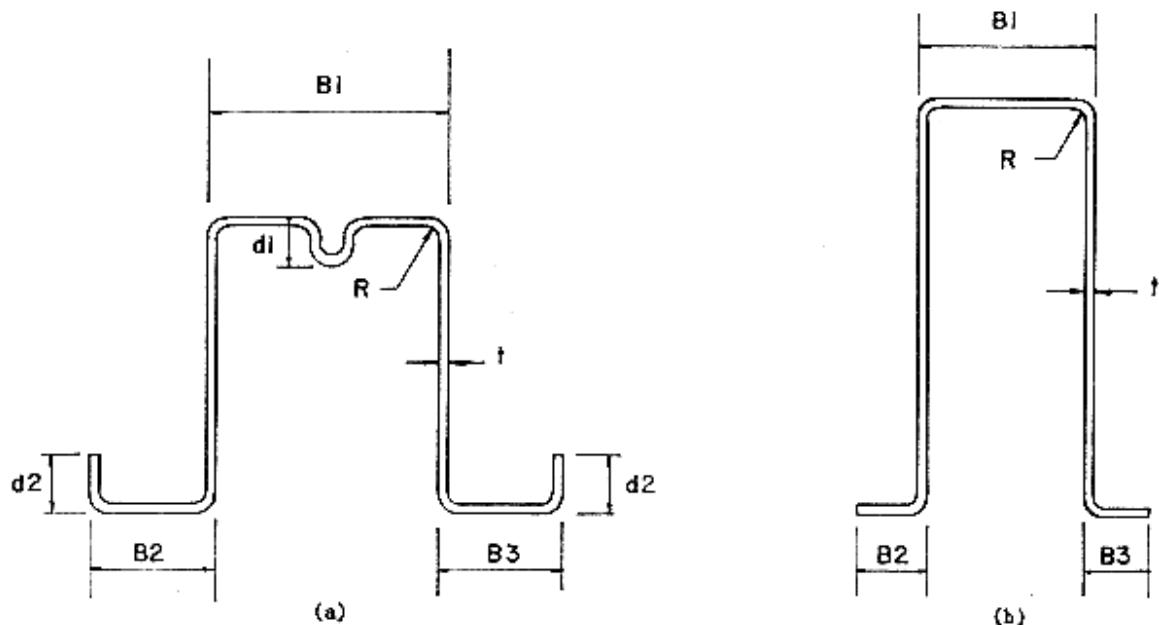


Figure A2: Hat Sections Tested at Cornell (1953)

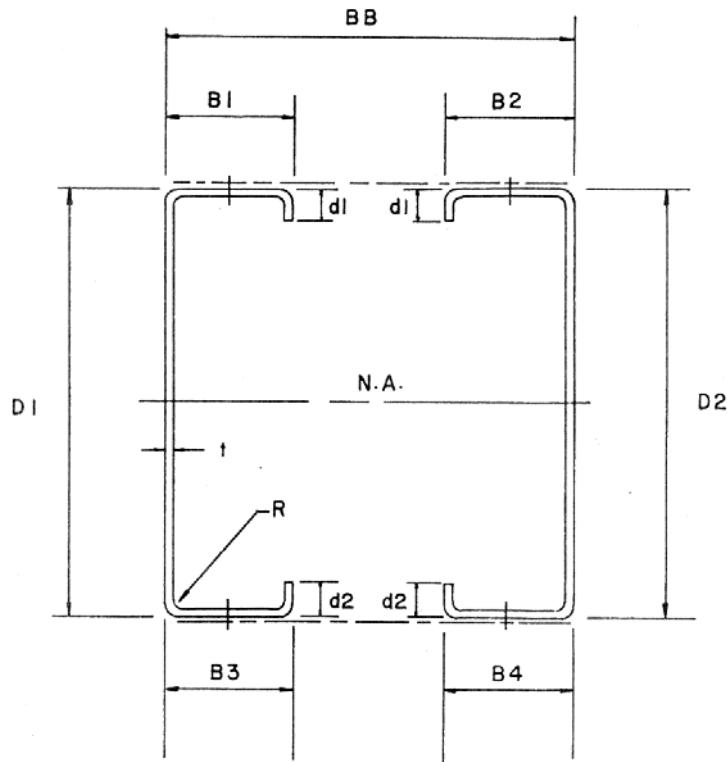


Figure A3: Channel C-sections Tested by Hettrakul and Yu (1978)

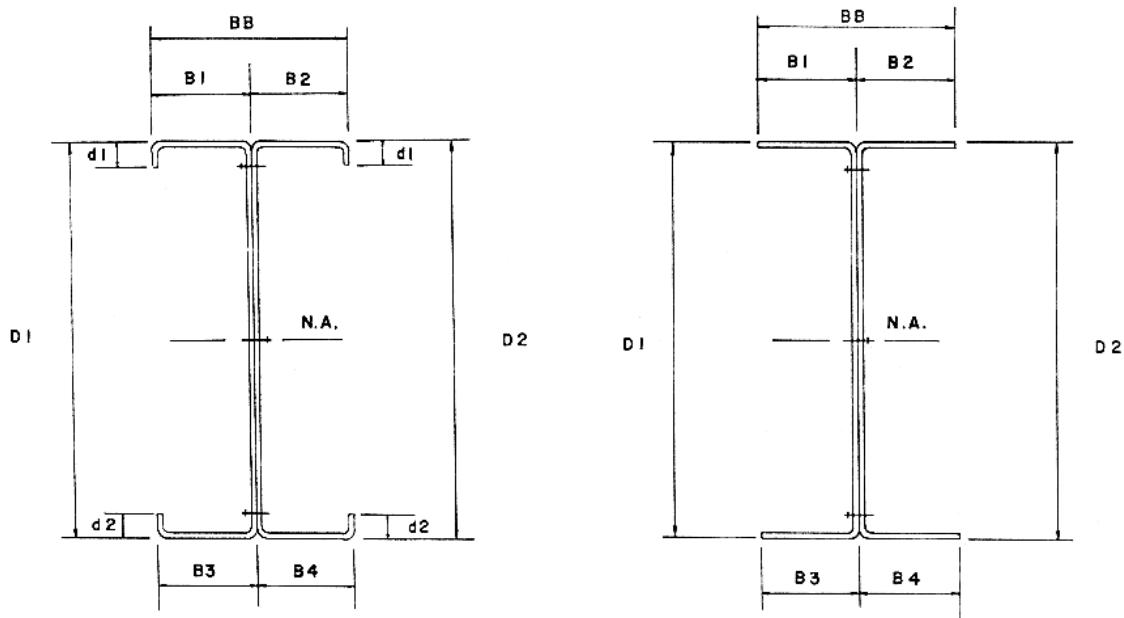


Figure A4: Stiffened and Unstiffened I-sections Tested by Hettrakul and Yu (1978)

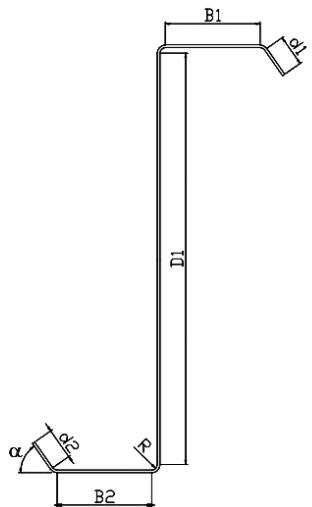


Figure A5: Z-sections Tested by LaBoube, Nunnery and Hodges (1994)

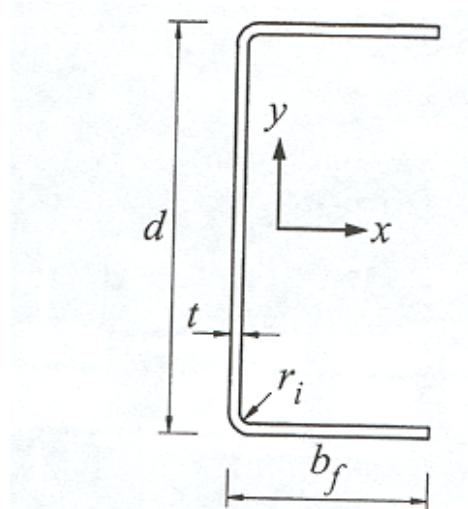


Figure A6: Unstiffened C-sections Tested by Young and Hancock (2000)

APPENDIX B - DATA AND COMPARISONS

Table B5: (Continued)

Specimen	t (in.)	R (in.)	N (in.)	B1 (in.)	B2 (in.)	B3 (in.)	B4 (in.)	d1 (in.)	d2 (in.)	D1 (in.)	D2 (in.)
I-3'-IOF-1	0.0460	0.0938	1.0	1.931	1.959	1.938	1.931	0.600	0.643	7.075	7.102
I-3'-IOF-2	0.0460	0.0938	1.0	1.937	1.935	1.927	1.973	0.608	0.607	7.112	7.065
I-3'-IOF-5	0.0460	0.0938	3.0	1.916	1.933	1.959	1.928	0.602	0.601	7.172	7.116
I-3'-IOF-6	0.0450	0.0938	3.0	1.924	1.952	1.922	1.928	0.605	0.590	7.106	7.108
I-5'-IOF-5	0.0600	0.0938	3.0	1.739	1.749	1.885	1.917	0.540	0.513	7.277	7.313
I-5'-IOF-6	0.0590	0.0938	3.0	1.735	1.745	1.880	1.895	0.528	0.520	7.292	7.337
I-6-IOF-1	0.0752	0.1016	1.0	1.750	1.751	1.931	1.936	0.509	0.544	7.286	7.321
I-6-IOF-2	0.0752	0.0938	1.0	1.750	1.750	1.928	1.934	0.509	0.522	7.287	7.325
I-6-IOF-5	0.0752	0.0938	3.0	1.789	1.783	1.929	1.931	0.493	0.529	7.398	7.399
I-6-IOF-6	0.0751	0.0938	3.0	1.768	1.772	1.930	1.934	0.496	0.517	7.381	7.420
I-6-IOF-7	0.0755	0.0859	3.0	1.768	1.762	1.930	1.927	0.497	0.521	7.417	7.391
I-6-IOF-8	0.0750	0.0938	3.0	1.767	1.770	1.929	1.933	0.502	0.515	7.390	7.430
I-6"-IOF-5	0.0460	0.0938	3.0	2.986	3.032	2.981	3.009	0.644	0.623	7.079	7.116
I-6"-IOF-6	0.0460	0.0938	3.0	2.938	2.940	2.951	2.946	0.683	0.673	6.931	6.974
I-12-IOF-6	0.0510	0.0938	3.0	1.497	1.481	1.513	1.522	0.589	0.617	7.444	7.491
I-12'-IOF-5	0.1080	0.1094	3.0	3.977	3.973	3.955	3.968	1.036	1.040	5.550	5.550
I-12'-IOF-6	0.1070	0.1094	3.0	3.935	3.991	3.963	3.975	1.038	1.038	5.492	5.507
I-16-IOF-1	0.0505	0.0938	1.0	2.516	2.524	2.536	2.513	0.565	0.582	4.056	4.031
I-16-IOF-2	0.0505	0.0938	1.0	2.519	2.516	2.534	2.524	0.617	0.556	4.029	4.041
I-16-IOF-5	0.0500	0.0938	3.0	2.493	2.513	2.509	2.499	0.611	0.556	3.978	4.006
I-16-IOF-6	0.0510	0.0938	3.0	2.524	2.502	2.535	2.508	0.616	0.600	3.980	3.974
I-13-IOF-1	0.0500	0.0625	4.0	1.792	1.807	1.832	1.793	0.610	0.614	4.007	4.020
I-13-IOF-2	0.0510	0.0625	4.0	1.789	1.770	1.803	1.788	0.590	0.624	4.007	4.023
I-4'-IOF-1	0.0497	0.0625	4.2	1.632	1.643	1.622	1.644	0.623	0.605	4.906	4.890
I-4'-IOF-2	0.0497	0.0625	4.2	1.620	1.629	1.631	1.639	0.602	0.636	4.889	4.898
I-2'-IOF-1	0.0497	0.0938	5.0	1.642	1.655	1.625	1.628	0.608	0.643	7.370	7.363
I-2'-IOF-2	0.0497	0.0938	5.0	1.648	1.657	1.625	1.622	0.612	0.660	7.347	7.350
I-3-IOF-1	0.0487	0.0938	6.0	1.625	1.636	1.633	1.633	0.652	0.609	9.806	9.828
I-3-IOF-1	0.0487	0.0938	6.0	1.662	1.630	1.629	1.647	0.625	0.619	9.788	9.826

Table B8: (Continued)

Specimen	Fy (ksi)	Pt (kip)	Pc (kip)	Mt (in-kip)	Mc (in-kip)	Pt/Pc	Mt/Mc	A/1.42 Eq.4	A'/1.32 Eq. 12
C200K1.0N37-a*	415	61.4	123	27.3	33.3	0.499	0.818	0.952	0.956
C200K1.0N37-b*	415	61.4	121	27.3	33.3	0.507	0.818	0.959	0.962
C200K1.5N37-a*	415	49.1	124	31.5	33.3	0.397	0.944	0.964	0.983
C200K1.5N37-b*	415	49.1	123	31.5	33.3	0.400	0.945	0.967	0.986
C300K0.5N90-a*	435	138.4	213	44.8	90.2	0.649	0.497	0.839	0.814
C300K0.5N90-b*	435	138.4	215	44.8	90.2	0.645	0.497	0.836	0.811
C300K1.0N90-a*	435	107.6	214	69.6	90.2	0.503	0.772	0.923	0.924
C300K1.0N90-b*	435	107.6	214	69.7	90.2	0.503	0.772	0.923	0.924
C300K0.5N45-a*	435	124.2	191	42.9	90.2	0.649	0.475	0.824	0.798
C300K0.5N45-b*	435	124.2	191	42.8	90.2	0.649	0.475	0.824	0.798
C300K1.0N45-a*	435	90.2	191	62.2	90.2	0.472	0.690	0.841	0.841
C300K1.0N45-b*	435	90.2	192	62.2	90.2	0.470	0.689	0.840	0.839

*Values for these sections are given in S.I. Units (MPa, kN, kN m).

Average: 1.025 1.004

Standard Deviation: 0.099 0.101

Coefficient of Variation: 0.097 0.101

$M_t/M_c \geq 0.3$

Table B9: Experimental and Calculated Results of Hat Sections – Cornell (1953)

Specimen	Fy (ksi)	Pt (kip)	Pc (kip)	Mt (in-kip)	Mc (in-kip)	Pt/Pc	Mt/Mc
1	33.6	1.93	1.98	5.79	17.0	0.976	0.341
2	34.5	2.26	2.35	6.78	17.6	0.960	0.386
3	30.8	2.62	2.32	7.86	15.6	1.130	0.504
4	35.2	1.67	1.78	5.00	18.1	0.936	0.276
5	32.4	1.87	1.89	5.60	16.9	0.985	0.331
6	33.8	2.02	2.15	6.06	17.4	0.939	0.348
7	54.9	2.85	3.54	8.55	28.9	0.805	0.296
8	55.2	3.34	4.06	10.0	29.2	0.822	0.343
9	54.0	4.10	4.42	12.0	28.6	0.927	0.421
10	52.0	2.79	3.08	8.37	29.0	0.905	0.289
11	54.7	3.10	3.52	9.30	29.9	0.881	0.311
12	54.8	3.54	3.91	10.6	29.9	0.905	0.355
15	31.1	2.21	1.88	13.3	39.3	1.177	0.338
21	53.8	3.75	3.74	22.5	70.3	1.003	0.320
27	31.8	1.78	1.59	16.0	42.0	1.119	0.381
32	53.3	2.66	2.84	24.0	66.5	0.937	0.360
33	52.4	2.88	3.15	25.9	66.5	0.912	0.389
45	55.0	3.07	3.18	36.8	102	0.963	0.359

Table B10: (Continued)

Specimen	Fy (ksi)	Pt (kip)	Mt (in-kip)	Pc (kip)	Mc (in-kip)	Pt/Pc	Mt/Mc	B/1.32 Eq. 5	B'/1.46 Eq. 13
I-3'-IOF-1	33.5	1.81	11.8	1.64	33.7	1.102	0.349	-	-
I-3'-IOF-2	33.5	1.85	12.0	1.64	34.1	1.126	0.353	-	-
I-3'-IOF-5	33.5	2.10	13.7	2.05	34.2	1.025	0.399	-	-
I-3'-IOF-6	33.5	2.32	15.1	1.96	32.8	1.178	0.459	1.080	1.025
I-5'-IOF-5	47.1	4.16	31.2	4.82	60.2	0.863	0.518	0.928	0.875
I-5'-IOF-6	47.1	4.00	30.0	4.66	58.5	0.858	0.513	0.921	0.868
I-6'-IOF-1	42.9	5.54	40.1	5.53	72.8	1.001	0.551	1.039	0.981
I-6'-IOF-2	42.9	5.40	39.2	5.58	72.9	0.967	0.537	1.008	0.951
I-6'-IOF-5	42.9	6.00	43.5	6.76	74.5	0.888	0.584	0.994	0.935
I-6'-IOF-6	42.9	6.49	38.2	6.74	74.1	0.962	0.516	0.989	0.934
I-6'-IOF-7	42.9	7.00	47.3	6.88	74.6	1.018	0.633	1.112	1.047
I-6'-IOF-8	42.9	6.98	47.1	6.72	74.2	1.038	0.635	1.126	1.060
I-6"-IOF-5	33.5	2.16	14.0	2.05	37.0	1.051	0.379	-	-
I-6"-IOF-6	33.5	2.32	15.1	2.05	37.4	1.129	0.403	1.007	0.957
I-12'-IOF-6	53.8	3.37	24.4	4.02	54.2	0.838	0.451	0.862	0.814
I-12'-IOF-5	45.7	12.1	72.4	14.2	122	0.850	0.592	0.976	0.918
I-12'-IOF-6	45.7	12.8	76.5	14.0	120	0.914	0.639	1.052	0.989
I-16'-IOF-1	53.8	2.73	13.0	3.19	28.4	0.856	0.457	0.878	0.829
I-16'-IOF-2	53.8	2.84	13.5	3.19	29.2	0.890	0.461	0.902	0.852
I-16'-IOF-5	53.8	3.53	16.8	3.88	28.3	0.909	0.592	1.014	0.954
I-16'-IOF-6	53.8	3.90	15.4	4.03	28.9	0.967	0.533	1.004	0.948
I-13'-IOF-1	53.8	3.62	14.5	4.39	26.7	0.823	0.542	0.922	0.868
I-13'-IOF-2	53.8	3.75	15.0	4.56	26.8	0.823	0.561	0.936	0.880
I-4'-IOF-1	36.9	2.92	17.4	3.00	24.4	0.971	0.715	1.145	1.075
I-4'-IOF-2	36.9	3.06	18.3	3.00	24.2	1.020	0.757	1.207	1.133
I-2'-IOF-1	36.9	3.10	24.4	2.97	39.2	1.042	0.622	1.119	1.054
I-2'-IOF-2	36.9	3.00	23.6	2.97	39.2	1.009	0.603	1.083	1.021
I-3'-IOF-1	36.9	3.37	29.5	3.00	53.1	1.123	0.556	1.119	1.058
I-3'-IOF-1	36.9	3.20	28.0	3.00	52.9	1.067	0.529	1.064	1.005

Average: 1.071 1.001

Standard Deviation: 0.101 0.091

Coefficient of Variation: 0.094 0.091

$M_t/M_c \geq 0.4$

**Table B11: Experimental and Calculated Results of Stiffened I-sections
(Winter & Pian (1946))**

Specimen	Fy (ksi)	Pt (kip)	Pc (kip)	Mt (in-kip)	Mc (in-kip)	Pt/Pc	Mt/Mc
1b-1-IOF	32.2	2.325	2.07	20.9	39.9	1.12	0.525
1c-1-IOF	32.2	2.60	2.24	23.4	39.9	1.16	0.587
2a-1-IOF	30.2	2.70	2.68	16.2	24.7	1.01	0.656
2b-1-IOF	30.2	3.25	2.68	8.13	24.7	1.21	0.329
2b-2-IOF	30.2	3.90	2.87	9.75	24.7	1.36	0.395
3-1-IOF	30.2	4.85	3.25	12.1	20.0	1.49	0.608
3-2-IOF	30.2	4.80	3.13	12.0	20.0	1.53	0.601
4b-1-IOF	30.2	4.35	3.34	17.4	45.8	1.30	0.380
5a-1-IOF	37.9	3.725	3.62	33.5	72.7	1.03	0.461
5b-1-IOF	37.9	4.10	4.11	36.9	72.7	0.996	0.508
5c-1-IOF	37.9	4.65	4.43	41.9	72.7	1.05	0.576
7a-1-IOF	35.8	5.70	4.94	51.3	93.7	1.15	0.547
7b-1-IOF	35.8	7.80	5.97	70.2	93.7	1.31	0.749
8-1-IOF	35.8	6.75	5.61	60.8	103	1.20	0.588
9b-1-IOF	35.1	10.3	8.84	25.8	46.9	1.16	0.549
10a-1-IOF	35.1	9.60	9.26	38.4	92.7	1.04	0.414
10a-2-IOF	35.1	12.0	9.78	48.0	92.7	1.23	0.518
10b-1-IOF	35.1	15.2	11.0	60.8	92.7	1.39	0.656
12-1-IOF	36.2	15.7	11.4	62.8	154	1.37	0.408
13a-1-IOF	35.7	15.3	14.1	38.3	57.2	1.08	0.669
13b-1-IOF	35.7	15.1	14.9	37.8	57.2	1.01	0.660
14a-1-IOF	33.1	16.5	15.7	66.0	114	1.05	0.579
14a-2-IOF	33.1	18.2	16.5	72.6	114	1.10	0.637
14b-1-IOF	33.1	21.2	17.8	84.6	114	1.19	0.743
15a-1-IOF	33.1	17.4	15.6	69.4	167	1.11	0.415
15a-2-IOF	33.1	20.3	16.4	81.0	167	1.24	0.484
15b-1-IOF	33.1	21.2	17.6	86.6	167	1.20	0.517
16a-1-IOF*	32.2	2.25	1.86	18.5	55.1	1.21	0.336
16b-1-IOF*	32.2	2.80	2.07	23.1	55.1	1.35	0.419
16c-1-IOF*	32.2	3.25	2.24	26.8	55.1	1.45	0.486
17a-1-IOF*	35.8	5.83	4.98	52.5	128	1.17	0.411
17b-1-IOF*	35.8	7.63	5.61	68.7	128	1.36	0.538
17c-1-IOF*	35.8	7.24	6.01	65.2	128	1.20	0.511

* Denotes single web sections

Table B12: Experimental and Calculated Results of Nested Z-sections
LaBoube, Nunnery and Hodges (1994)

Specimen	t (in.)	h (in.)	R (in.)	N (in.)	Fy (ksi)	Pt (kip)	Mt (in-kip)	Pc (kip)	Mc (in-kip)	Pt/Pc	Mt/Mc	C/1.65 Eq. 6	C'/1.65 Eq. 14
3	0.058	7.830	0.250	4.99	50.9	2.39	84.1	2.36	98.2	1.010	0.856	1.039	1.045
4	0.058	7.830	0.250	4.99	50.9	2.37	83.4	2.36	98.2	1.001	0.849	1.030	1.036
5	0.061	7.747	0.312	5.00	56.8	2.91	85.0	2.65	108	1.096	0.787	1.041	1.048
6	0.061	7.747	0.312	5.00	56.8	2.70	78.8	2.65	108	1.016	0.730	0.966	0.972
7	0.061	7.747	0.312	5.00	56.8	3.36	58.0	2.65	108	1.267	0.537	0.978	0.986
8	0.061	7.747	0.312	5.00	56.8	3.36	58.0	2.65	108	1.267	0.537	0.978	0.986
9	0.061	7.747	0.312	5.00	56.8	2.37	97.6	2.65	108	0.892	0.904	1.007	1.013
10	0.061	7.747	0.312	5.00	56.8	2.35	96.7	2.65	108	0.884	0.896	0.999	1.004
11*	0.071	7.739	0.312	4.97	52.4	3.24	75.3	3.10	115	1.045	0.655	0.935	0.942
13*	0.071	7.739	0.312	4.97	52.4	2.75	96.9	3.10	115	0.886	0.842	0.967	0.972
14*	0.071	7.739	0.312	4.97	52.4	2.82	99.4	3.10	115	0.909	0.864	0.992	0.997
28	0.061	7.747	0.312	5.00	56.8	3.03	70.5	2.65	108	1.143	0.653	0.984	0.991
29	0.061	7.747	0.312	5.00	56.8	2.84	83.1	2.65	108	1.071	0.769	1.018	1.025
30	0.061	7.747	0.312	5.00	56.8	2.41	84.8	2.65	108	0.907	0.785	0.943	0.949

Average: 0.991 0.998

Standard Deviation: 0.033 0.033

Coefficient of Variation: 0.034 0.033

* These sections were composed of nested Z-sections of two different thicknesses, 0.061 in. and 0.071 in.

Table B13: Experimental and Calculated Results of I-sections (Unstiffened Flanges)
Hetrakul and Yu (1978)

Specimen	Fy (ksi)	Pt (kip)	Mt (in-kip)	Pc (kip)	Mc (in-kip)	Pt/Pc	Mt/Mc
I-U-17-IOF-5	36.3	2.57	14.1	2.74	13.8	0.936	1.025
I-U-17-IOF-6	36.3	2.50	13.8	2.74	13.8	0.912	0.994

APPENDIX C - INTERACTION DIAGRAMS

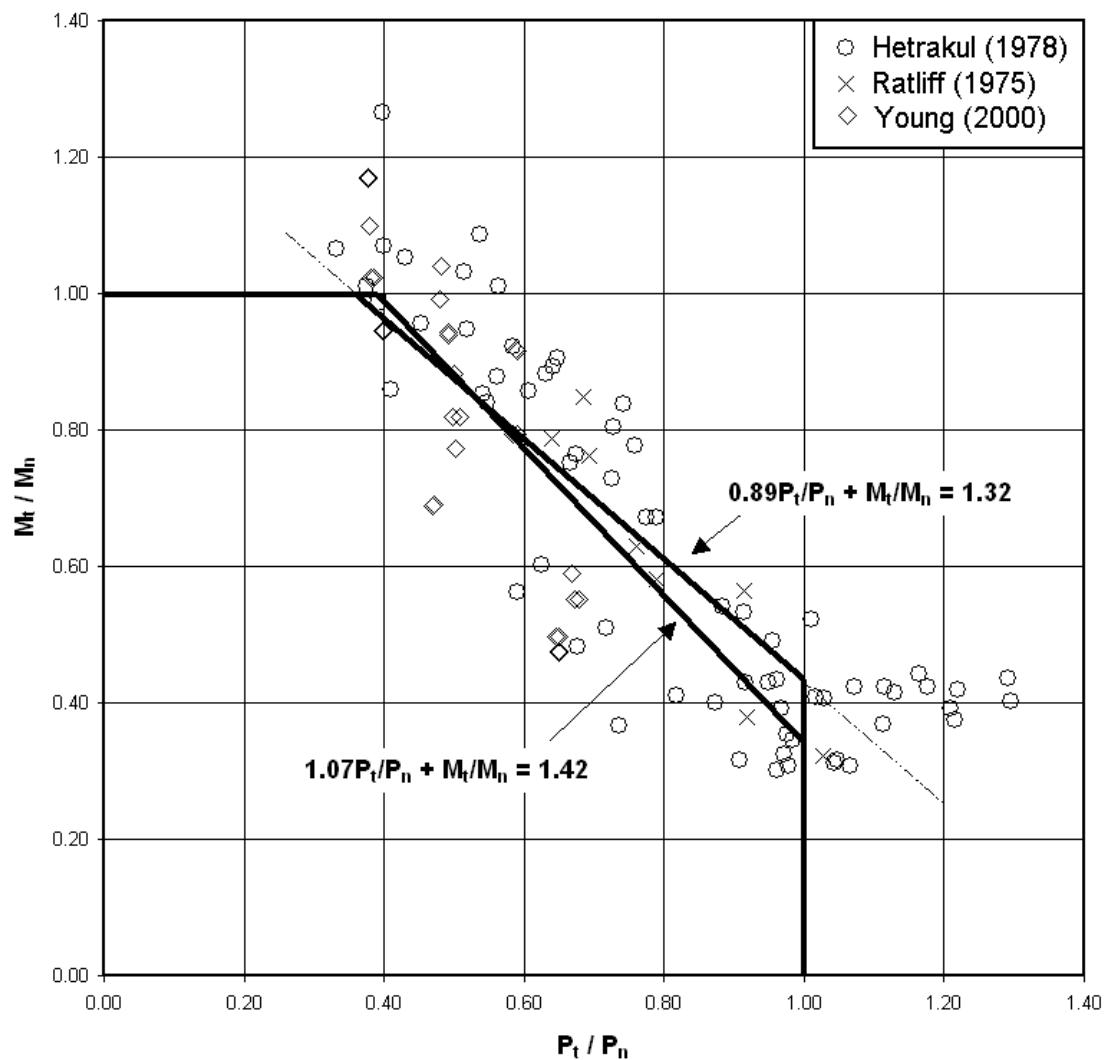


Figure C1: Interaction Diagram of Channel C-sections

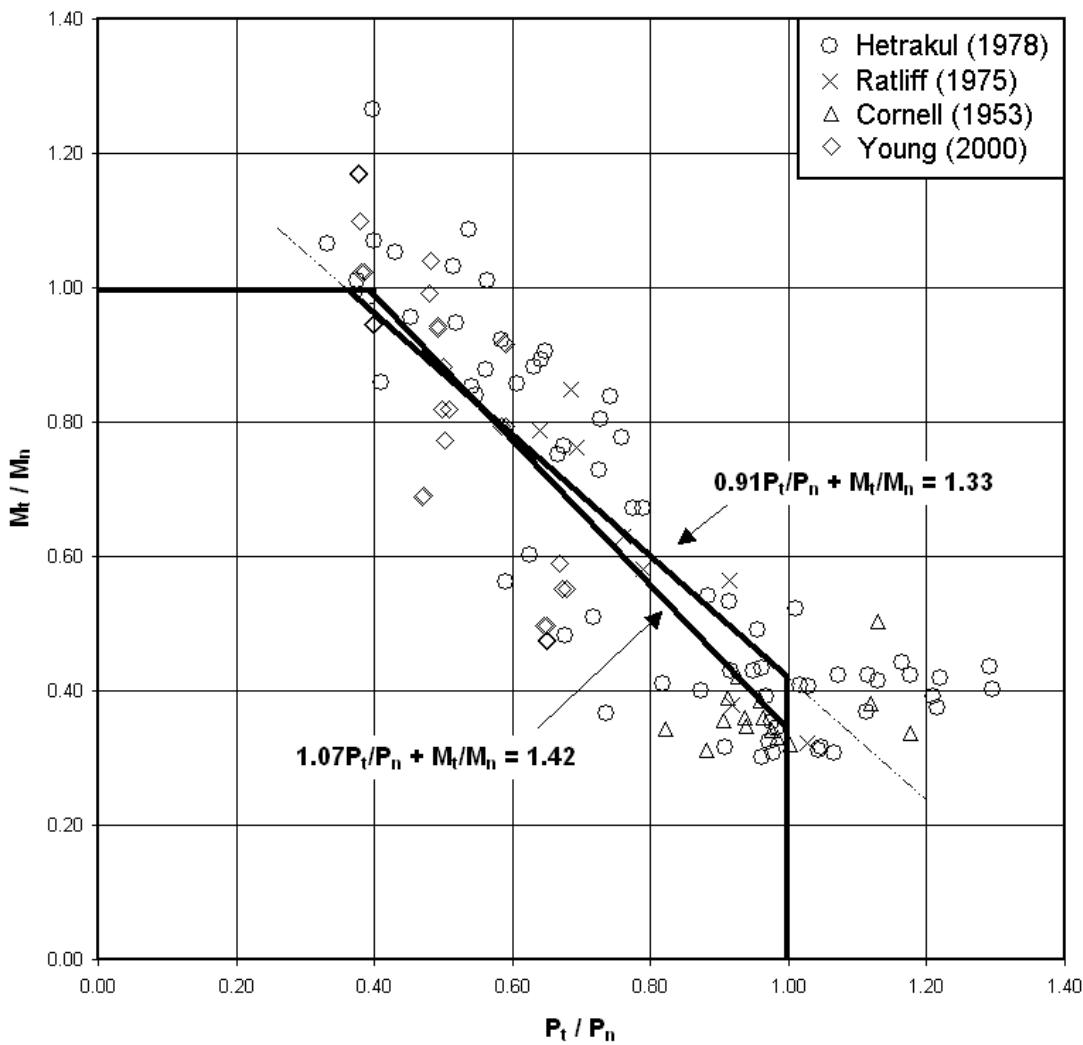


Figure C2: Interaction Diagram of C- and Hat Sections

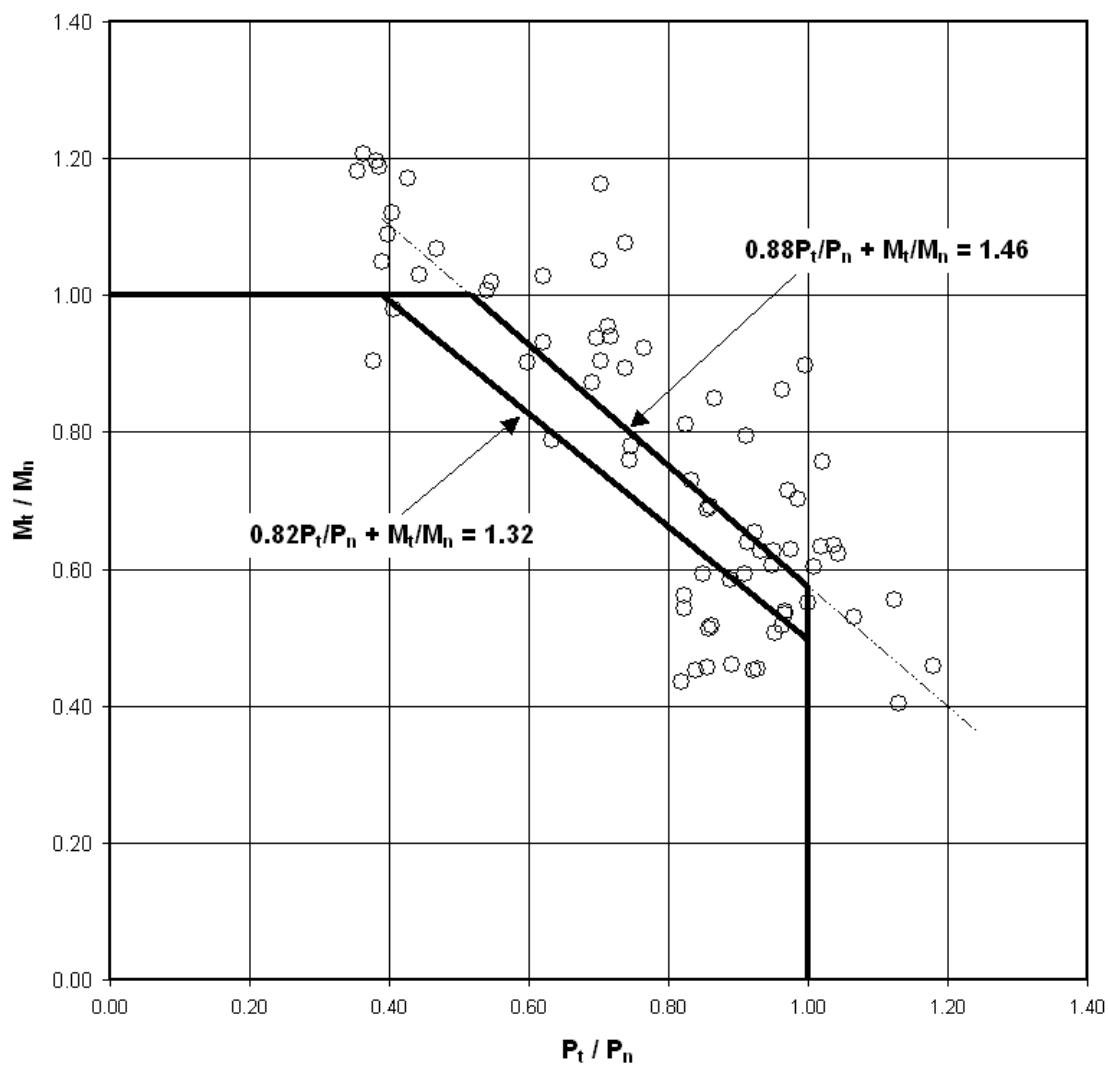


Figure C3: Interaction Diagram of I-sections (Hetrakul 1978)

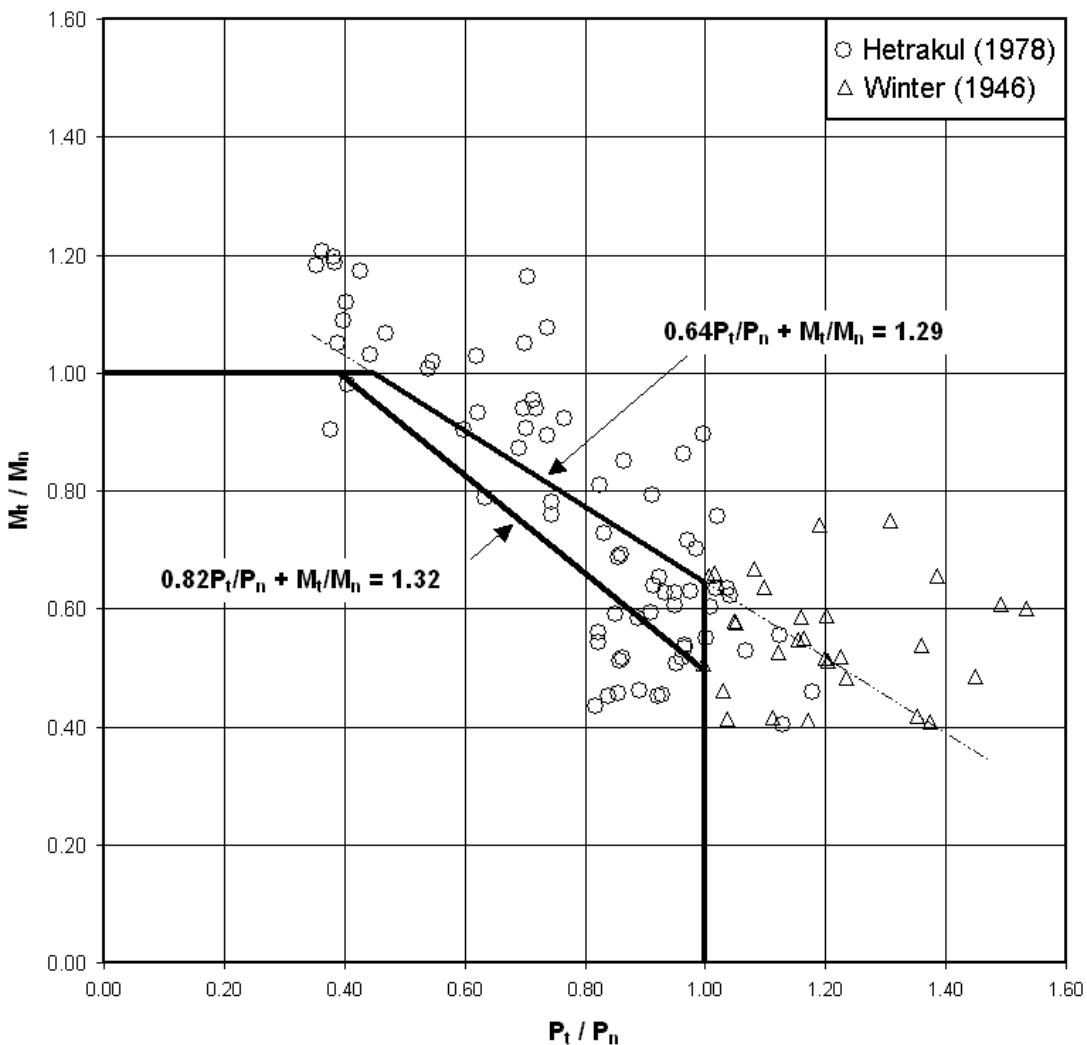


Figure C4: Interaction Diagram of I-sections

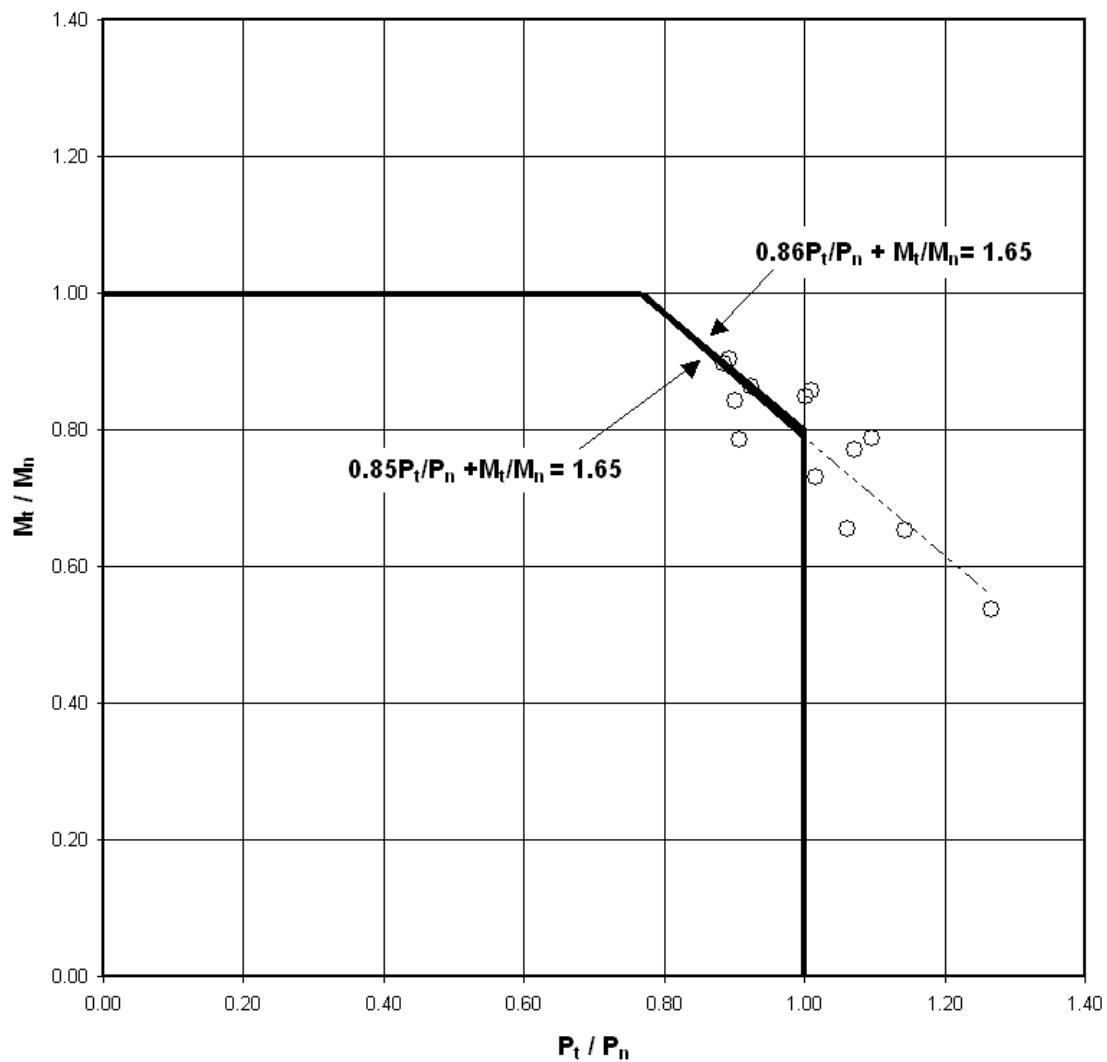


Figure C5: Interaction Diagram of Nested Z-sections (LaBoube 1994)



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