

# **A Design Approach for Complex Stiffeners**

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**A DESIGN APPROACH FOR COMPLEX STIFFENERS**

FINAL REPORT

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A PROJECT SPONSORED BY  
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# A Design Approach for Complex Stiffeners

## 1 Introduction

This report presents a design approach for laterally braced cold-formed steel flexural members with edge stiffened flanges other than simple lips. The objectives of the research are as follows:

- To study the feasibility of using the design method for flexural members given by Schafer (1997), Schafer and Peköz (1999) to design these complex stiffeners and then compare the results with the finite element method and AISI (1996). (Chapters 2-4)
- To use a computer program, CU-EWA, developed to perform parameter studies for cross section optimization. (Chapter 5-6)
- To conduct a preliminary study on a two-point bending experiment setup as preparation for future testing by doing a full finite element simulation of the experimental arrangement and by making comparisons with previous physical test results. (Chapter 7)
- To study an alternative approach for introducing the initial geometric imperfection by using the stochastic process to randomly generate signals for the imperfection geometric shape instead of introducing the initial geometric imperfection by superimposing the eigenmodes. (Chapter 8)

The following is a summary of the design procedures for flexural members given by Schafer (1997), Schafer and Peköz (1999). The design procedures are based on the need for the integration of the distortional mode into the design procedure. Two behavioral phenomena must be considered. First, the distortional mode has less post-buckling capacity than the local mode. Second, the distortional mode has the ability to control failure even when it occurs at a higher critical stress than the local mode. A design method incorporating these phenomena is needed to provide an integrated approach to strength prediction involving local and distortional buckling. For consistency with the existing cold-form steel design specifications an effective width approach was undertaken. Effective section properties are based on effective widths,  $b$ .

$$b = \rho w \quad (1)$$

where  $w$  is the actual element width and post-buckling reduction factor,  $\rho$ , is

$$\rho = (1 - 0.22/\lambda)/\lambda \text{ for } \lambda > 0.673 \text{ otherwise } \rho = 1. \quad (2)$$

where

$$\lambda = \sqrt{f_y / f_{cr}} \quad (3)$$

In order to properly integrate distortional buckling, reduced post-buckling capacity in the distortional mode and the ability of the distortional mode to control the failure mechanism even when at a higher buckling stress than the local mode must be incorporated. Therefore, the critical buckling stress of the element was defined by comparing the local buckling stress and distortional buckling stress to determine the governing mode as follows:

$$(f_{cr}) = \min[(f_{cr})_{local}, R_d (f_{cr})_{dist.}] \quad (4)$$

$$R_d = \min\left(1, \frac{1.17}{\lambda_d + 1} + 0.3\right) \text{ where } \lambda_d = \sqrt{f_y / (f_{cr})_{dist.}} \quad (5)$$

$R_d$  reflects the reduced strength in mechanisms associated with distortional failures. For  $R_d < 1$  this method provides an additional reduction on the post-buckling capacity. Further, the method allows the distortional mode to control situations where the distortional buckling stress is greater than the local buckling stress. Thus,  $R_d$  provides a framework for solving the problem of predicting the failure mode and reducing the post-buckling capacity in the distortional mode. Schafer (1997), Schafer and Peköz (1999) developed the expression for  $R_d$  based on post-buckling capacity as shown in Figure 1.2, 1.3 and from the experimental results of Hancock et al. (1994). With  $f_{cr}$  of the element known the effective width of each element may readily be determined and the effective section properties generated.

To investigate the post-buckling behavior and develop the expression for  $R_d$  the authors analyzed an isolated flange-stiffener model as shown in Figure 1.1 rather than using the full section. Two types of imperfections, local and distortional mode, are superposed to give the initial geometric imperfections. The magnitude of the imperfection is selected based on the statistical summary provided in Schafer (1997), Schafer and Peköz (1999). Then the ultimate strength of these isolated flanges is found for different

magnitudes of imperfection. Two maximum imperfection magnitudes, one at 25% and the other at 75% probability of exceedance, are used. The percent differences in the strength are used to measure the imperfection sensitivity.

$$\frac{(f_u)_{75\% imp.} - (f_u)_{25\% imp.}}{\frac{1}{2}((f_u)_{75\% imp.} + (f_u)_{25\% imp.})} \times 100\% \quad (6)$$

The error bars in Figure 1.2 show the range of strengths predicted for imperfections varying over the central 50% portion of expected imperfection magnitudes. The greater the error bars, the greater the imperfection sensitivity. A contour plot of this imperfection sensitivity statistic is shown in Figure 1.3. Stocky members tending to failure in the distortional mode have the highest sensitivity.

The design approach given by Schafer (1997), Schafer and Peköz (1999) was developed based on simple lips edge stiffeners. Therefore, before adopting this approach for other types of stiffeners verification on the reduction factor for distortional stress,  $R_d$  is needed. This is done by creating a post-buckling capacity graph and imperfection sensitivity contour plot on the types of stiffeners in interest for comparison with the original Figure 1.2 and 1.3. In this research 4 types of complex stiffeners shown in Figure 2.1 are studied.

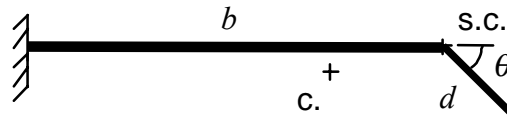


Figure 1.1 Isolated Flange-Stiffener Model

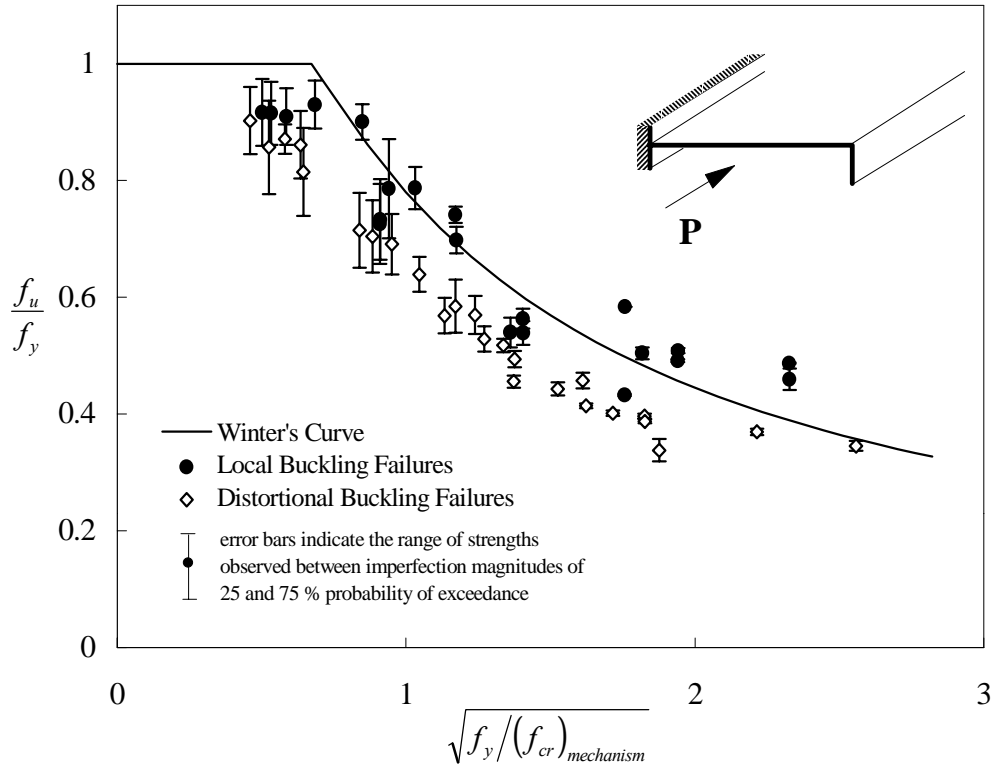


Figure 1.2 Post-Buckling Capacity of Edge Stiffened Flanges

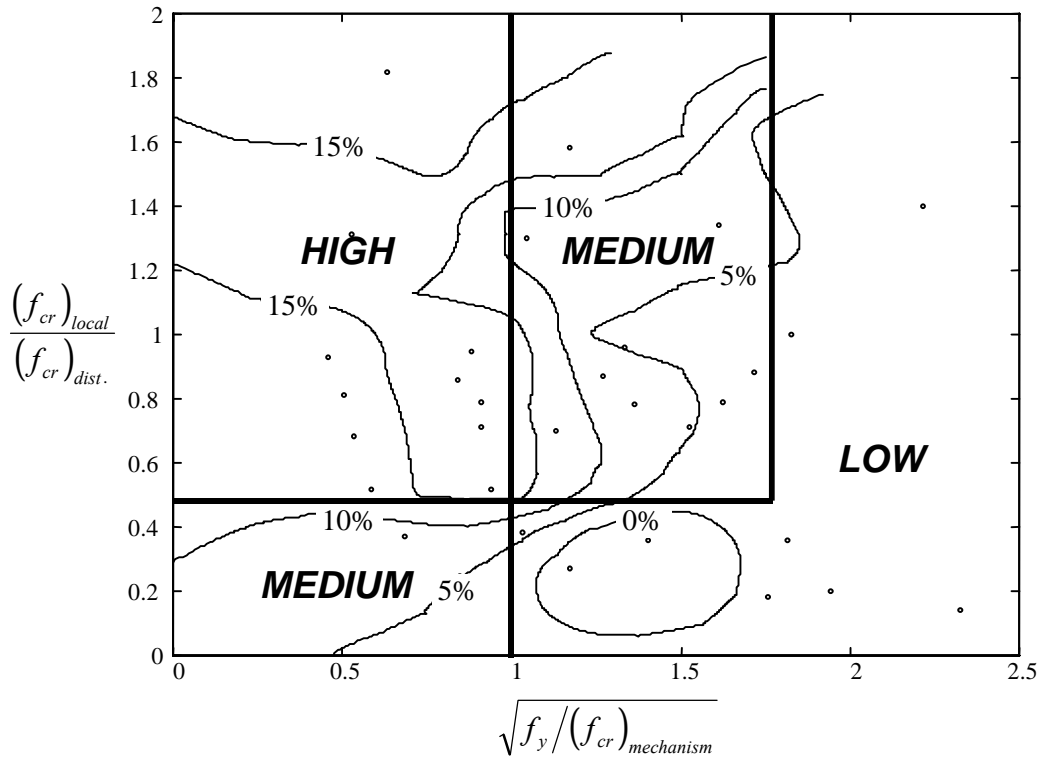


Figure 1.3 Imperfection Sensitivity of Edge Stiffened Flanges

## 2 Design Approach for Complex Stiffeners

Similar to the design approach given by Schafer (1997), Schafer and Peköz (1999) to account for the distortional buckling an evaluation of the reduction factor for distortional stress,  $R_d$  based on experimental and FEM studies must be considered. FEM studies were carried out for 4 types of complex stiffeners shown in Figure 2.1.

### 2.1 Finite Element Study

In order to study the post-buckling behavior of complex stiffened elements nonlinear FEM analyses are performed using ABAQUS. To study only the complex stiffened element behavior an idealization of the boundary conditions at the web/flange junction are made by restraining all degrees of freedom except for the translation along the length. Roller supports are used at both ends. To avoid localized failure at the ends, the uniform load has been distributed to the first row of elements. Boundary conditions are shown in Figure 2.2. The material model used is elastic-plastic with strain hardening and  $f_y = 347$  MPa. Residual stress is also included with a 30% yield stress throughout the thickness in the longitudinal direction. The residual stresses are assumed tension on the outside and compression in the inside of the section.

Initial geometric imperfection is introduced by superimposing the eigenmodes for the local and distortional buckling shown in Figure 2.3. The magnitude of the imperfection is selected based on the statistical summary provided in Schafer (1997), Schafer and Peköz (1999). Four types of complex stiffeners shown in Figure 2.1 have been studied. The length of the model is selected by using the length that would give the least buckling strength in the distortional mode, which is obtained, by using the Finite Strip Method CUFSM. Table 2.1 summarizes the geometry of the members. An imperfection sensitivity study has been performed for each type of stiffener. Figure 2.4, 2.6, 2.8, 2.10 and contour plot in Figure 2.5, 2.7, 2.9, 2.11 show the results. For each type of stiffener a total of 42 models were investigated.

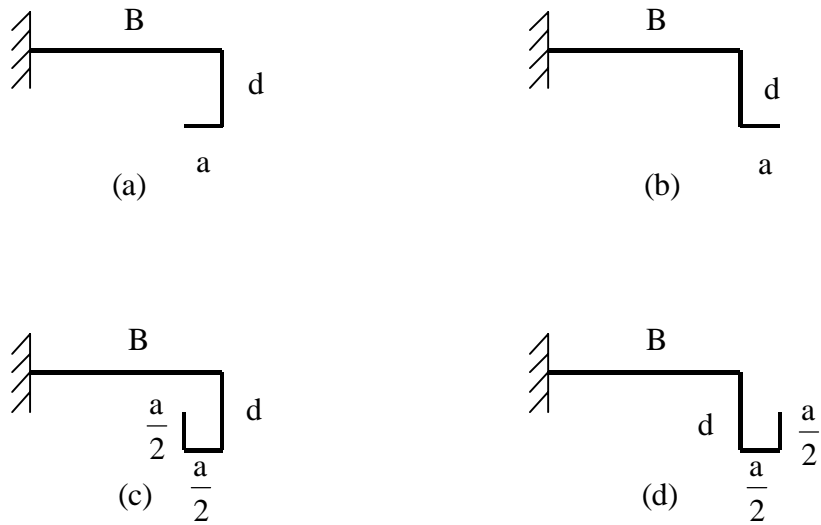


Figure 2.1 Isolated Flange-Complex Stiffener Model (a) Inside Angled Stiffener (b) Outside Angled Stiffener (c) Inside Hooked Stiffener (d) Outside Hooked Stiffener

Table 2.1 Geometry of Members \*

B	d	a
25	6.25	0,3.125,6.25
	12.5	0,6.25,12.5
50	6.25	0,3.125,6.25
	12.5	0,6.25,12.5
	25	0,12.5,25
75	6.25	0,3.125,6.25
	12.5	0,6.25,12.5
	25	0,12.5,25
	37.5	0,18.75,37.5
100	6.25	0,3.125,6.25
	12.5	0,6.25,12.5
	25	0,12.5,25
	37.5	0,18.75,37.5
	50	0,25,50

\* Thickness = 1 mm in all cases

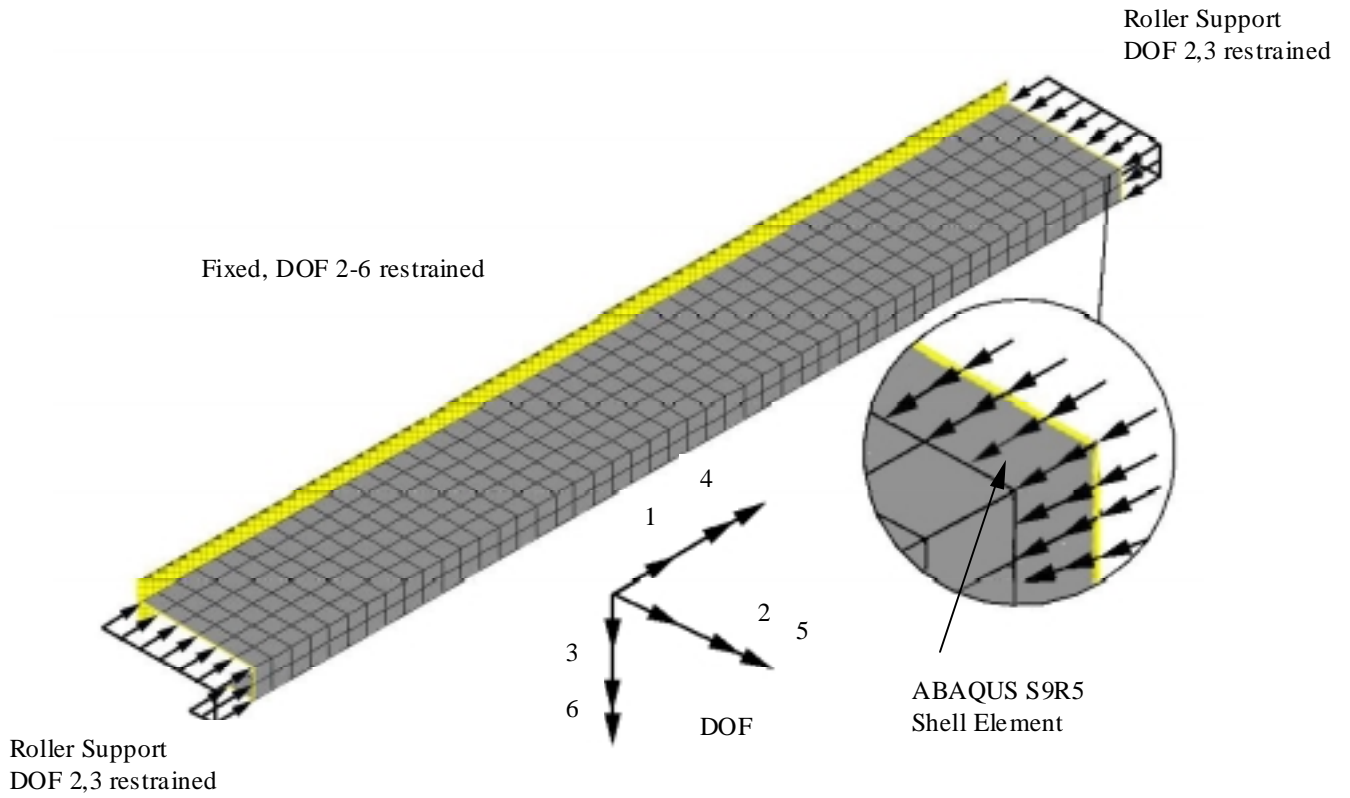


Figure 2.2 Boundary and Loading Condition

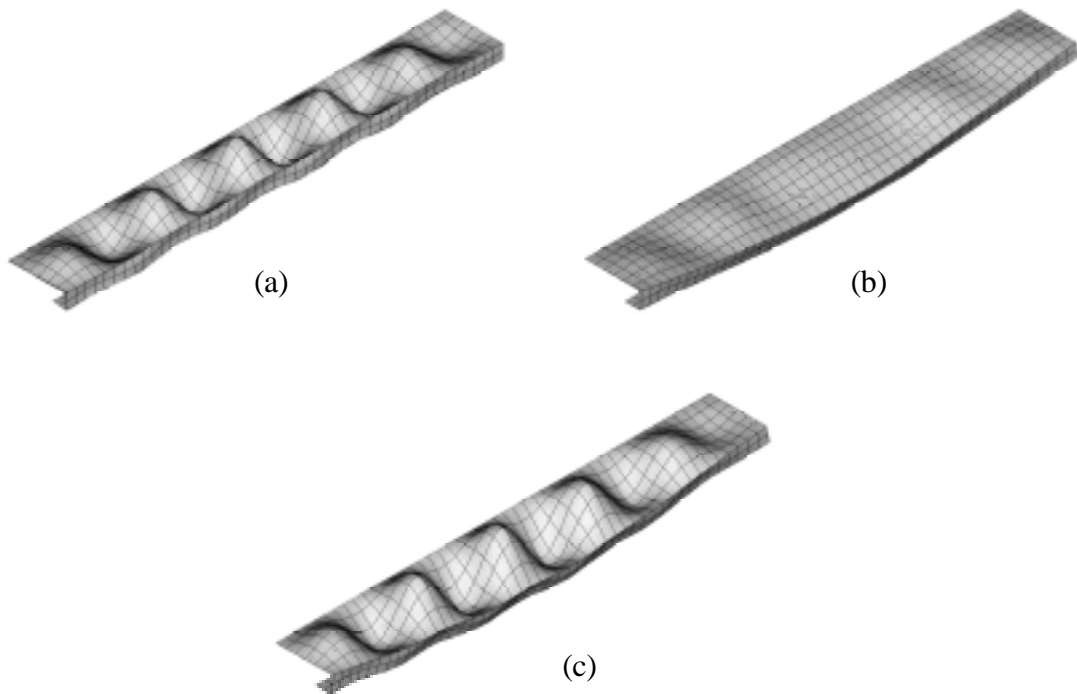


Figure 2.3 (a) Local Buckling (b) Distortional Buckling (c) Geometric Imperfection

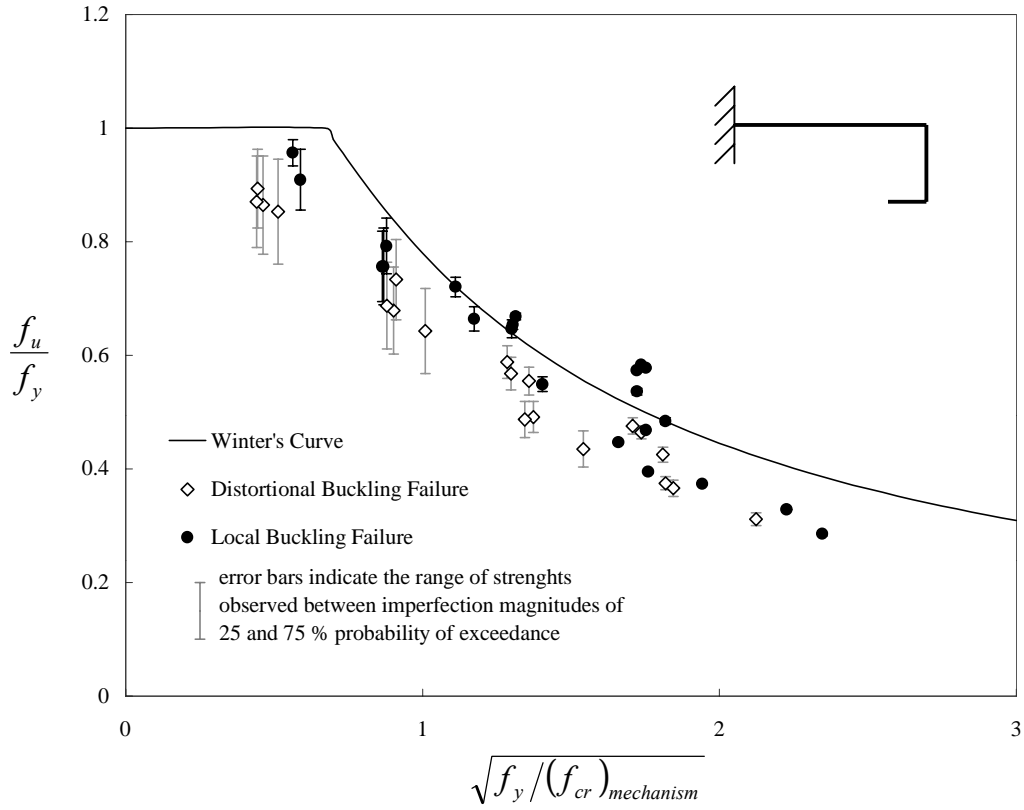


Figure 2.4 Post-Buckling Capacity of Inside Angled Stiffener

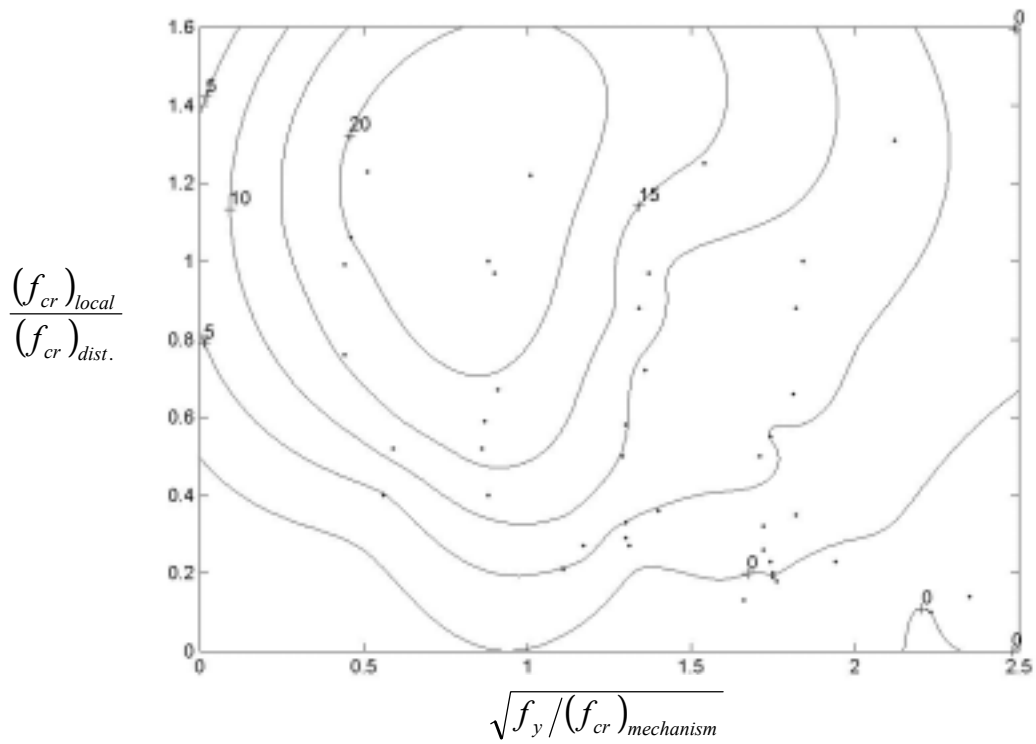


Figure 2.5 Imperfection Sensitivity of Inside Angled Stiffener



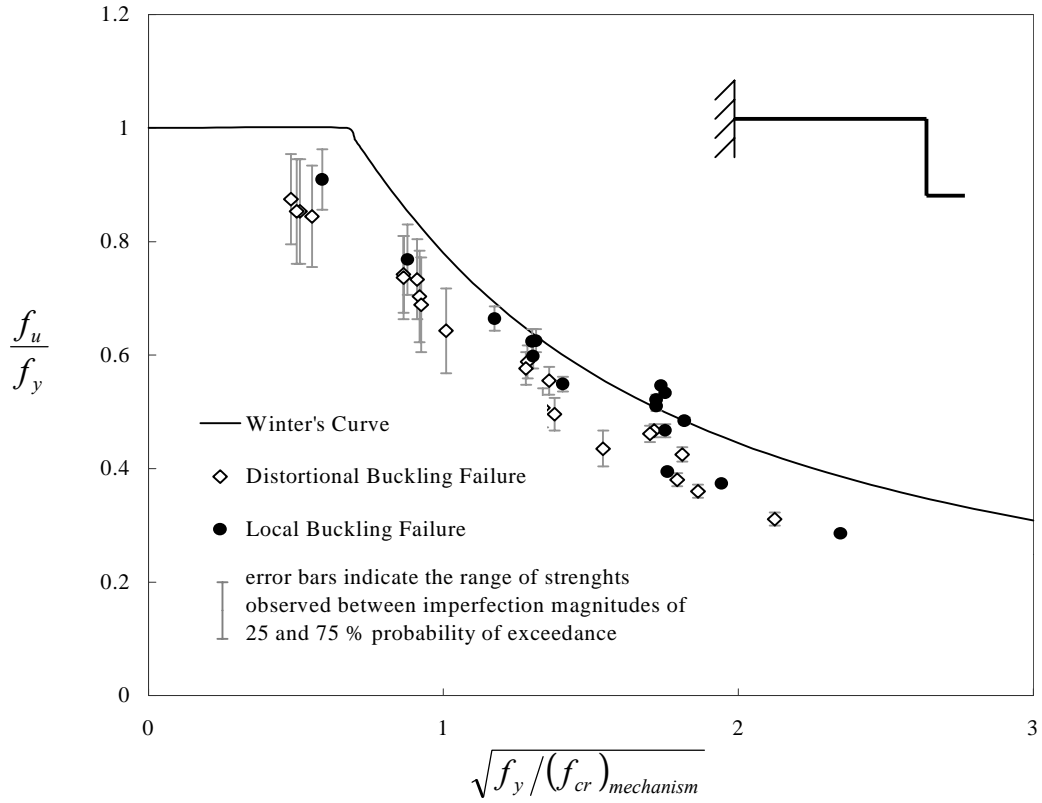


Figure 2.6 Post-Buckling Capacity of Outside Angled Stiffener

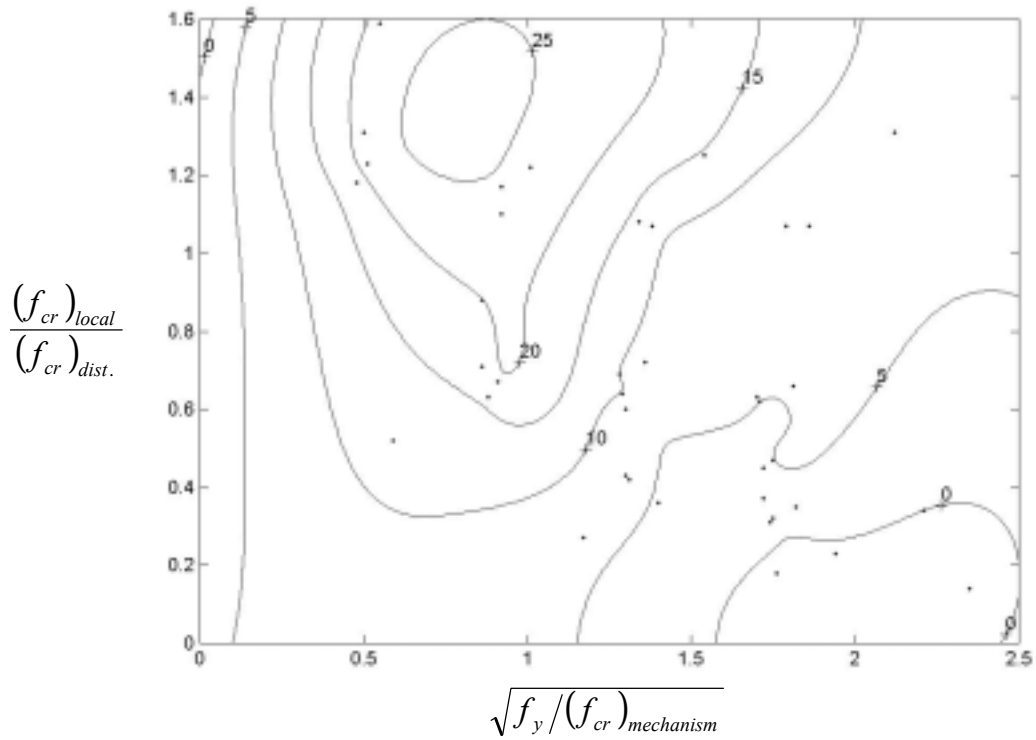


Figure 2.7 Imperfection Sensitivity of Outside Angled Stiffener

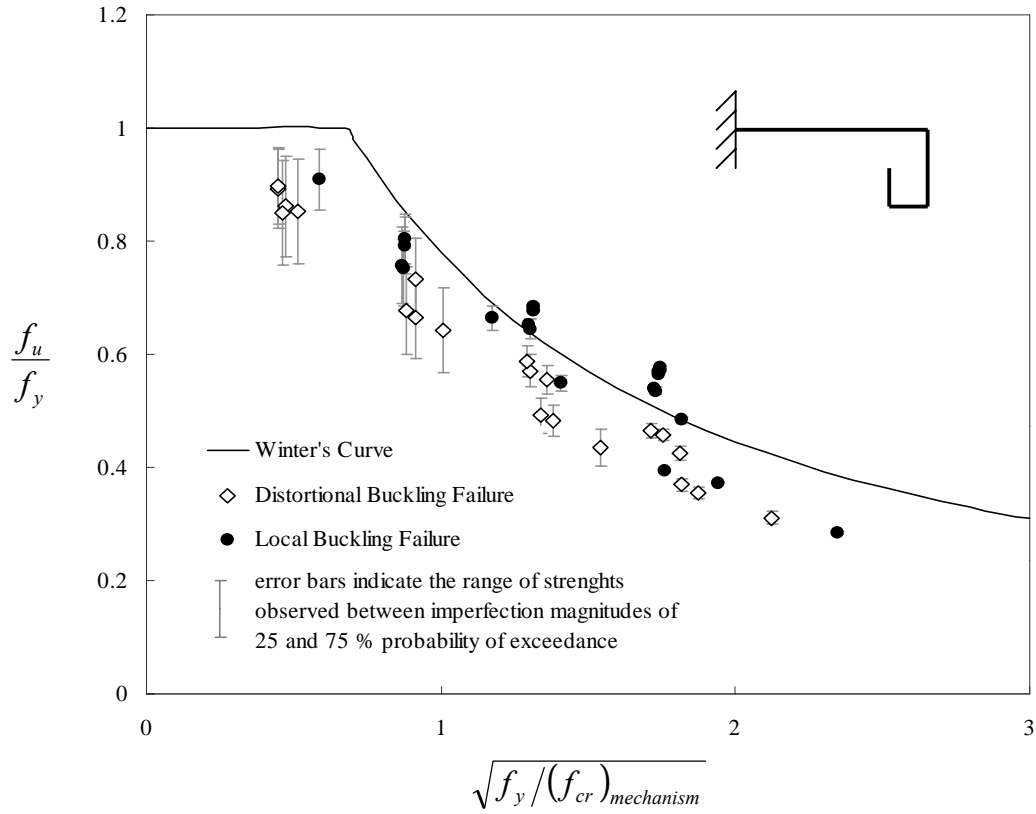


Figure 2.8 Post-Buckling Capacity of Inside Hooked Stiffener

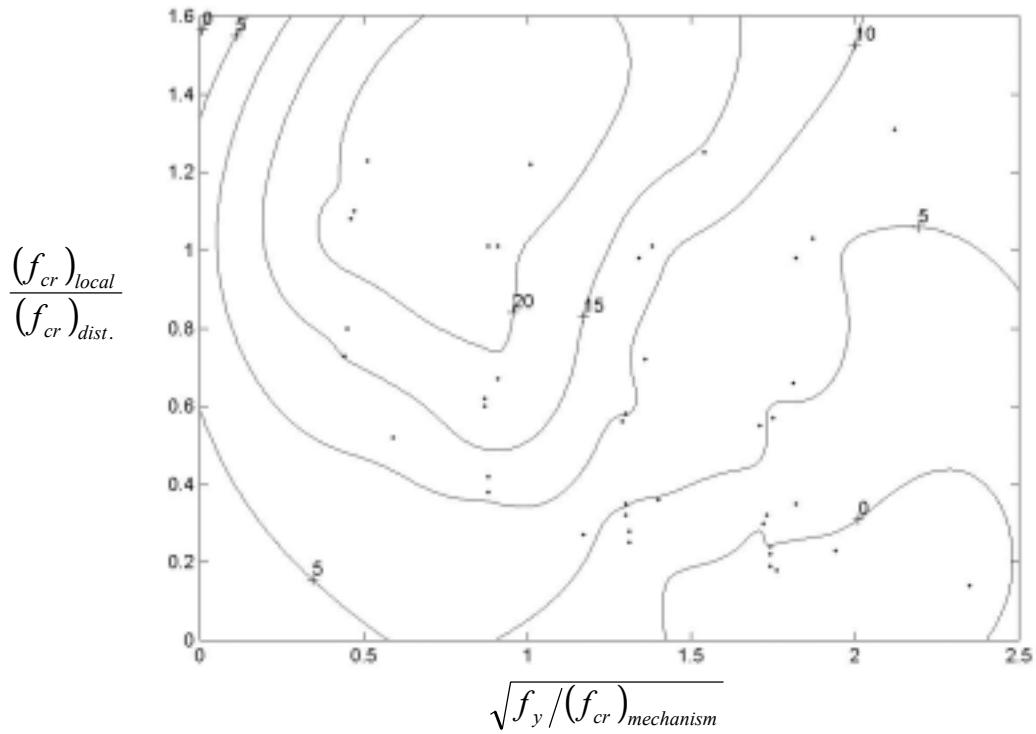


Figure 2.9 Imperfection Sensitivity of Inside Hooked Stiffener

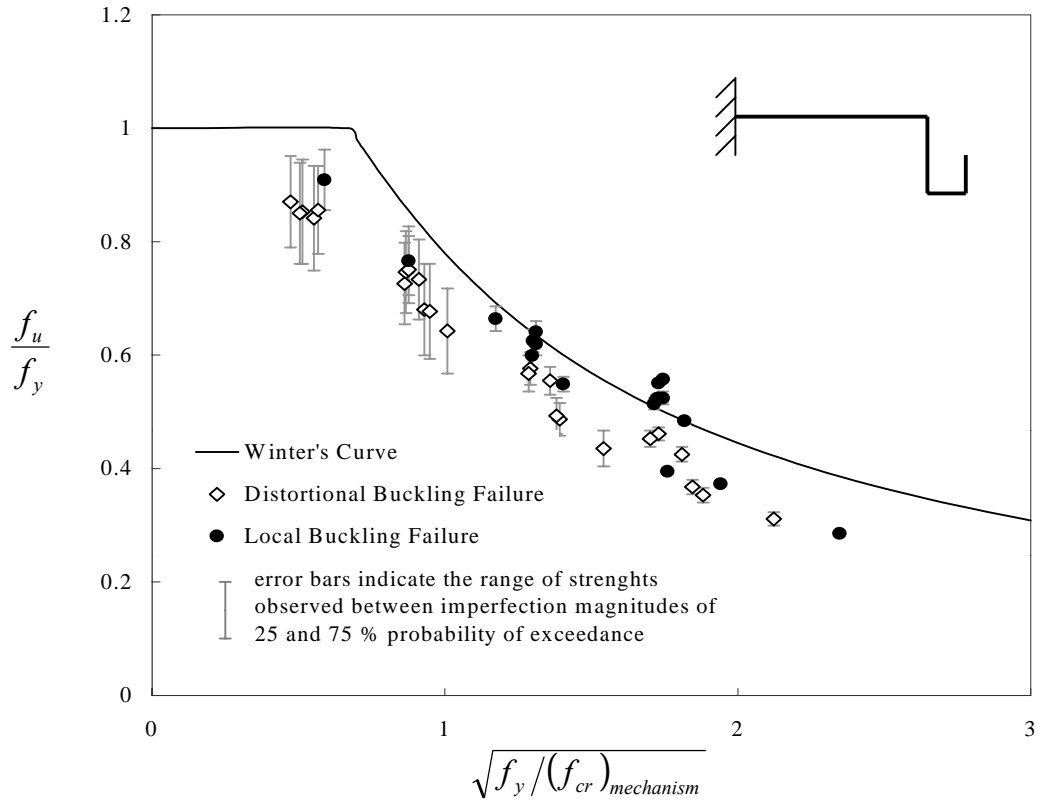


Figure 2.10 Post-Buckling Capacity of Outside Hooked Stiffener

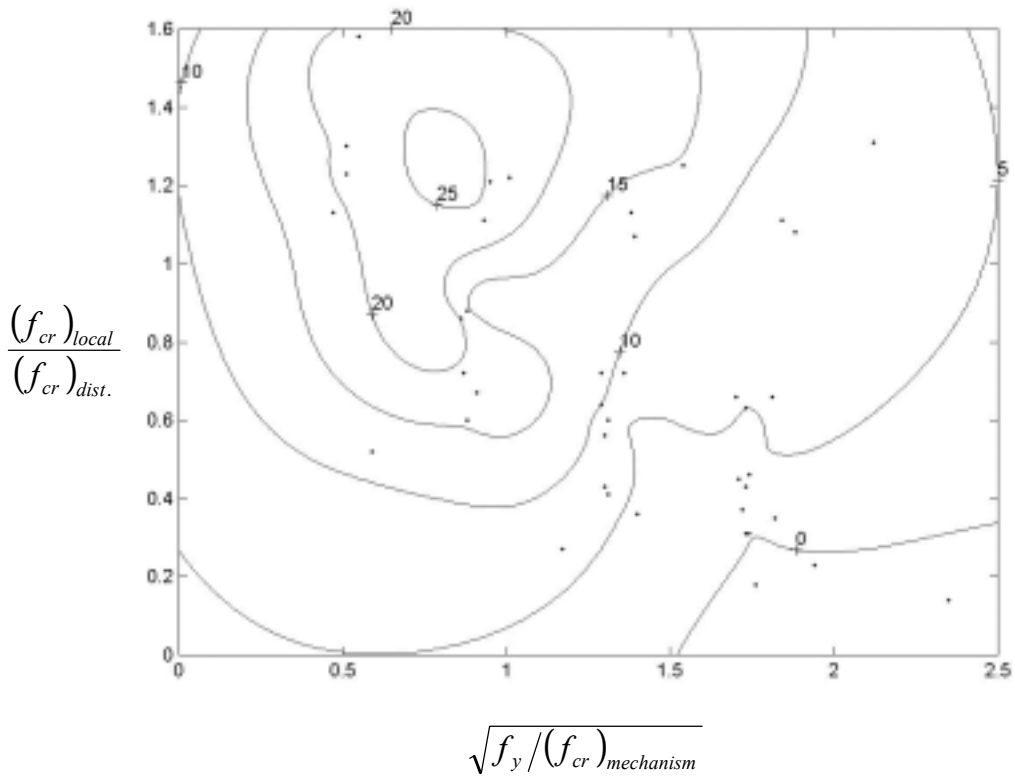


Figure 2.11 Imperfection Sensitivity of Outside Hooked Stiffener

## **2.2 Conclusion and Verification of the Proposed Design Approach**

It can be seen that the imperfection sensitivity contour plots are similar to those obtained by edge stiffened flanges. Therefore, the reduction factor for distortional stress,  $R_d$  is expected to be similar to Schafer (1997), Schafer and Peköz (1999).

## **3 Parametric Studies on Laterally Braced Flexural Members with Complex Stiffeners**

Two Z-section parameter studies are carried out for different types of stiffeners to compare the moment capacity determined by FEM, AISI (1996) approach and the proposed design approach for flexural members given by Schafer (1997), Schafer and Peköz (1999). First parameter study consists of different Z-section geometries with all cross sections having the same thickness while the second parameter study uses one standard Z-section but varies thickness. The cross sections selected for both these parameter studies are intend to cover a wide range of slendernesses and maintain the cross section area between each types of stiffeners to compare the efficiency.

### **3.1 Z-Section Parameter Study for Constant Thickness**

The parameter study is done by changing different widths of the web, flange and stiffeners for five types of stiffeners simple lip, inside angled, outside angled, inside hooked, and outside hooked stiffeners. All cross sections have the same thickness. Figure 3.1 and Table 3.1 summarize the geometry of the members. Local and distortional buckling stresses obtained by finite strip method are used for the proposed design approach for flexural members given by Schafer (1997), Schafer and Peköz (1999). Results for each type of stiffeners for this parameter study are shown in Table 3.2, 3.3, 3.4, 3.5, and 3.6. Table 3.7, 3.8, 3.9,3.10 and Figure 3.3, 3.4, 3.5 summarize the results for different approaches.

### 3.2 Finite Element Modeling Assumptions

The web/flange junction is restrained only for the translation degree of freedom perpendicular to the length to brace the member laterally. Roller supports are used at both ends. To avoid localized failure at the ends, the constant moment modeled by nodal loads is distributed to the first row of elements. Boundary and loading conditions are shown in Figure 3.2 (a). The material model used is elastic-plastic with strain hardening and  $f_y = 345$  MPa. Residual stress throughout the thickness in the longitudinal direction is assumed to be 30% of the yield stress in the flange and 40% of the yield stress in the web. The residual stresses are also assumed to be tension on the outside and compression in the inside of the section.

Initial geometric imperfections are introduced by superimposing the eigenmodes for the local and distortional buckling shown in Figure 3.2. Three different imperfection magnitudes 100% (no imperfections), 75% and 25% probability of exceedance based on the statistical summary provided in Schafer (1997), Schafer and Peköz (1999) are used for each model. The length of the model is selected by using three half-wave lengths of the distortional mode that gives the least buckling strength. The wavelengths are obtained using the Finite Strip Method.

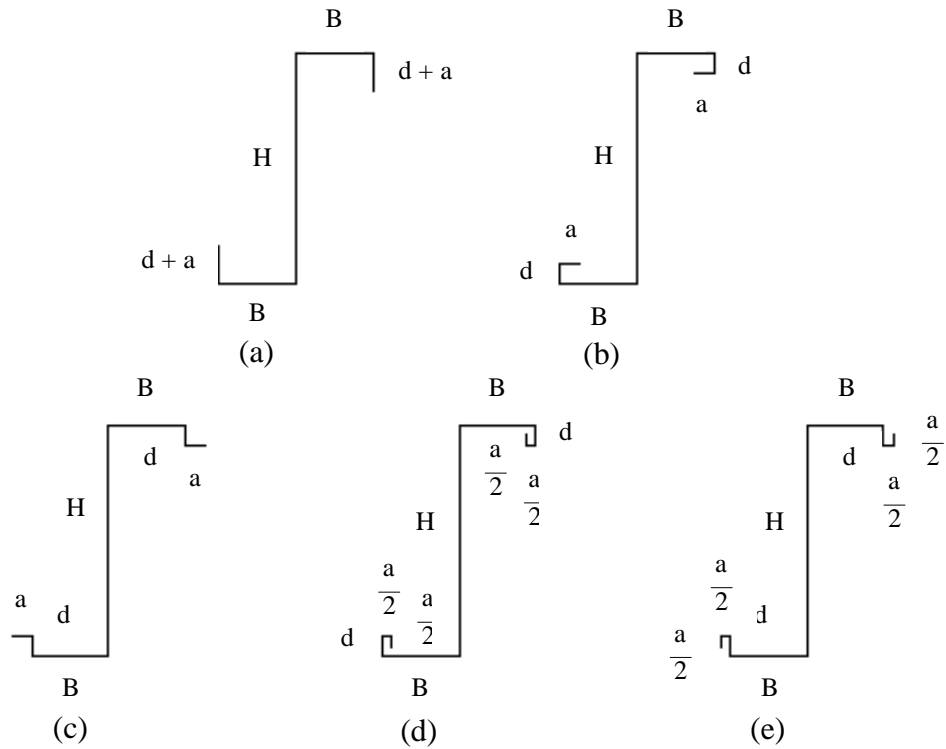


Figure 3.1 Z-section with (a) Simple Lip Stiffener (b) Inside Angled Stiffener (c) Outside Angled Stiffener (d) Inside Hooked Stiffener (e) Outside Hooked Stiffener

Table 3.1 Summary of Models Geometry \*

Model	Dimensions								
	H (mm)	B (mm)	d (mm)	a (mm)	Length, L (mm)				
					Simple Lip	Inside Angled	Outside Angled	Inside Hooked	Outside Hooked
1	50	25	6.25	3.125	630	600	570	600	570
2	50	25	6.25	6.25	780	690	570	630	570
3	100	25	6.25	3.125	720	690	660	660	630
4	100	25	6.25	6.25	900	780	660	720	660
5	100	50	6.25	3.125	1170	1080	1050	1050	1050
6	100	50	6.25	6.25	1410	1200	1110	1140	1080
7	100	50	12.5	6.25	1830	1740	1620	1680	1620
8	100	50	12.5	12.5	2190	1950	1650	1830	1650
9	150	25	6.25	3.125	720	660	660	660	630
10	150	25	6.25	6.25	930	780	690	750	690
11	150	50	6.25	3.125	1230	1170	1140	1140	1110
12	150	50	6.25	6.25	1500	1290	1200	1230	1170
13	150	50	12.5	6.25	1980	1860	1770	1800	1740
14	150	50	12.5	12.5	2370	2100	1800	1980	1770

\* Thickness = 1 mm in all cases

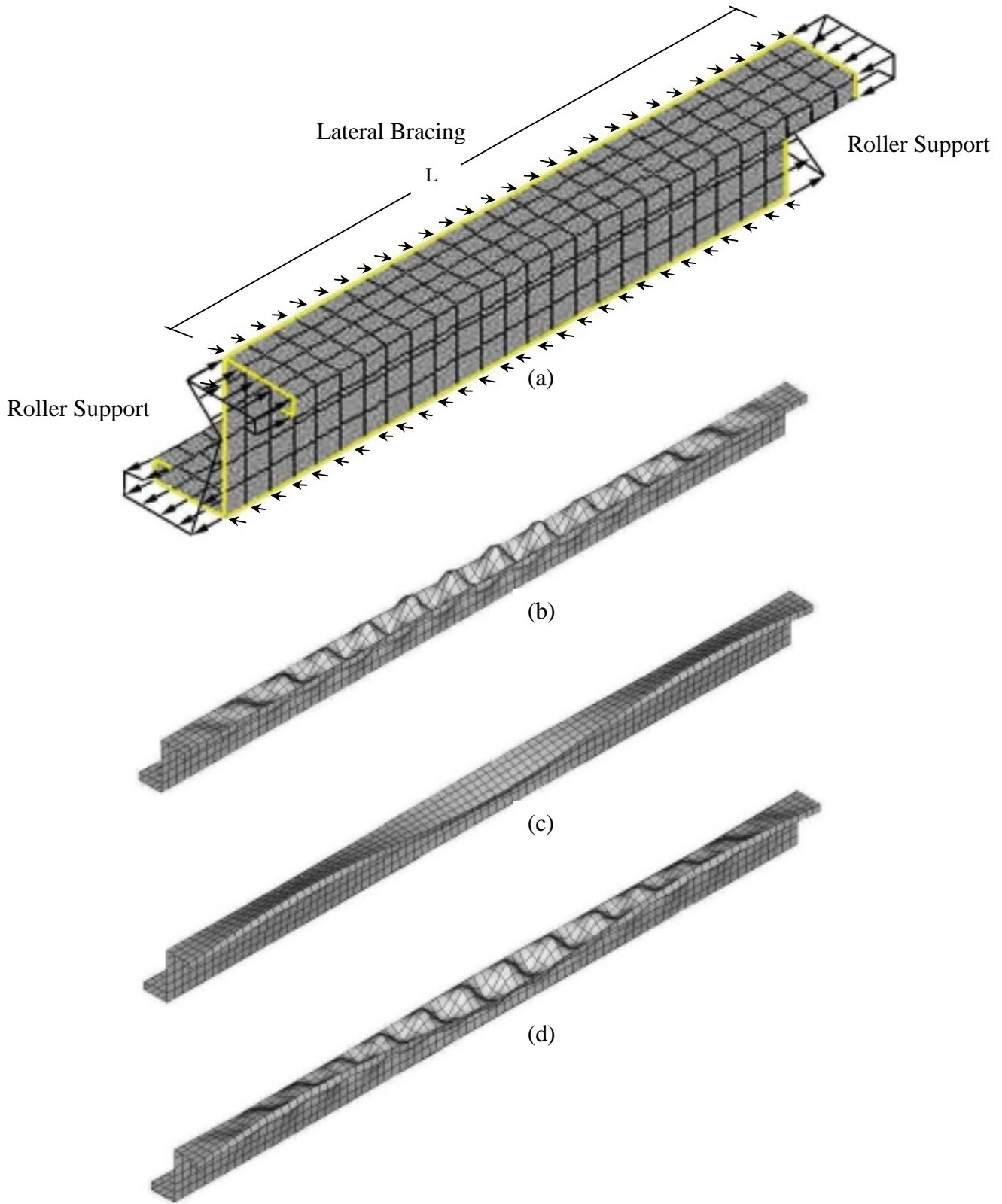


Figure 3.2 (a) Boundary and Loading Condition (b) Local Buckling (c) Distortional Buckling (d) Initial Geometric Imperfection

Table 3.2 Moment Capacity for Z-section with Simple Lip Stiffener

Model	Finite Strip Analysis					FEM		AISI	PRO	M <sub>PRO</sub> /M <sub>AISI</sub>	M <sub>FEM</sub> /M <sub>AISI</sub>	M <sub>FEM</sub> /M <sub>PRO</sub>	
	(fcr)d (MPa)	Rd	Rd(fcr)d (MPa)	(fcr)l (MPa)	fcr (MPa)	fu100 fu75 fu25	Mu100 Mu75 Mu25	M <sub>AISI</sub> (N-m)	M <sub>PRO</sub> (N-m)		imp 100% imp 75% imp 25%	imp 100% imp 75% imp 25%	
1	942	1	942	1346	942	376 362 309	745 718 613	670	670	1.00	1.11 1.07 0.92	1.11 1.07 0.92	
2	1132	1	1132	1263	1132	380 373 340	771 757 691	682	687	1.01	1.13 1.11 1.01	1.12 1.10 1.01	
3	583	0.961	560	573	560	373 337 314	1840 1665 1551	1687	1574	0.93	1.09 0.99 0.92	1.17 1.06 0.99	
4	659	0.979	645	573	573	380 355 322	1947 1823 1650	1734	1645	0.95	1.12 1.05 0.95	1.18 1.11 1.00	
5	245	0.835	205	345	205	265 226 222	1969 1684 1650	1945	1740	0.89	1.01 0.87 0.85	1.13 0.97 0.95	
6	317	0.873	277	348	277	309 285 242	2356 2174 1845	2185	1976	0.90	1.08 1.00 0.84	1.19 1.10 0.93	
7	445	0.922	410	342	342	314 289 252	2489 2292 1994	2190	2201	1.01	1.14 1.05 0.91	1.13 1.04 0.91	
8	542	0.951	515	317	317	307 299 276	2492 2430 2243	2120	2226	1.05	1.18 1.15 1.06	1.12 1.09 1.01	
9	331	0.879	291	266	266	324 311 291	2828 2719 2544	2879	2282	0.79	0.98 0.94 0.88	1.24 1.19 1.12	
10	380	0.899	341	266	266	330 321 308	2998 2916 2794	2950	2372	0.80	1.02 0.99 0.95	1.26 1.23 1.18	
11	204	0.808	165	238	165	252 221 218	3149 2766 2719	3010	2742	0.91	1.05 0.92 0.90	1.15 1.01 0.99	
12	255	0.841	215	238	215	252 246 219	3227 3160 2815	3468	3081	0.89	0.93 0.91 0.81	1.05 1.03 0.91	
13	342	0.884	302	238	238	293 278 250	3930 3736 3356	3465	3345	0.97	1.13 1.08 0.97	1.17 1.12 1.00	
14	400	0.907	363	235	235	278 271 253	3857 3766 3508	3321	3466	1.04	1.16 1.13 1.06	1.11 1.09 1.01	
									mean		0.94	1.08	1.15
									mean			1.02	1.09
									mean			0.93	0.99

(fcr)d – Distortional Buckling critical stress

Rd – Reduction factor for Distortional Buckling critical stress

(fcr)l – Local Buckling critical stress

fcr – Controlling critical stress

fu100, fu75, fu25 – Ultimate stress for imperfection magnitudes 100%, 75% and 25% probability of exceedance

Mu100, Mu75, Mu25 – Ultimate moment for imperfection magnitudes 100%, 75% and 25% probability of exceedance

M<sub>FEM</sub>, M<sub>AISI</sub>, M<sub>PRO</sub> – Nominal Moment from FEM, AISI and proposed design approach for flexural members



Table 3.3 Moment Capacity for Z-section with Inside Angled Stiffener

Model	Finite Strip Analysis					FEM		AISI	PRO	M <sub>PRO</sub> /M <sub>AISI</sub>	M <sub>FEM</sub> /M <sub>AISI</sub>	M <sub>FEM</sub> /M <sub>PRO</sub>	
	(fcr)d (MPa)	Rd	Rd(fcr)d (MPa)	(fcr)l (MPa)	fcr (MPa)	fu fu25 fu75	Mu Mu25 Mu75	M <sub>AISI</sub> (N-m)	M <sub>PRO</sub> (N-m)		imp 100% imp 75% imp 25%	imp 100% imp 75% imp 25%	
1	869	1.000	869	1418	869	373 355 299	744 709 596	675	675	1.00	1.10 1.05 0.88	1.10 1.05 0.88	
2	980	1.000	980	1504	980	373 362 305	776 755 635	705	705	1.00	1.10 1.07 0.90	1.10 1.07 0.90	
3	549	0.953	522	566	522	362 332 306	1795 1647 1517	1693	1554	0.92	1.06 0.97 0.90	1.16 1.06 0.98	
4	600	0.966	580	569	569	369 337 300	1918 1753 1558	1775	1666	0.94	1.08 0.99 0.88	1.15 1.05 0.94	
5	221	0.820	181	342	181	264 209 206	1968 1556 1536	2027	1678	0.83	0.97 0.77 0.76	1.17 0.93 0.92	
6	245	0.835	205	355	205	274 238 204	2105 1832 1569	2131	1805	0.85	0.99 0.86 0.74	1.17 1.02 0.87	
7	407	0.909	370	386	370	315 296 251	2517 2360 2002	2372	2263	0.95	1.06 0.99 0.84	1.11 1.04 0.88	
8	466	0.929	433	390	390	308 296 247	2570 2470 2059	2490	2407	0.97	1.03 0.99 0.83	1.07 1.03 0.86	
9	311	0.870	270	262	262	320 296 275	2800 2595 2404	2885	2283	0.79	0.97 0.90 0.83	1.23 1.14 1.05	
10	345	0.885	305	262	262	325 312 294	2977 2850 2693	3020	2384	0.79	0.99 0.94 0.89	1.25 1.20 1.13	
11	183	0.793	145	235	145	252 217 215	3145 2709 2688	3184	2638	0.83	0.99 0.85 0.84	1.19 1.03 1.02	
12	200	0.806	161	238	161	236 207 190	3044 2666 2452	3365	2819	0.84	0.90 0.79 0.73	1.08 0.95 0.87	
13	317	0.873	277	245	245	301 286 261	4056 3856 3521	3842	3386	0.88	1.06 1.00 0.92	1.20 1.14 1.04	
14	359	0.891	320	248	248	302 292 262	4261 4130 3696	4052	3569	0.88	1.05 1.02 0.91	1.19 1.16 1.04	
									mean		0.89	1.03	1.15
									mean			0.94	1.06
									mean			0.85	0.95

Table 3.4 Moment Capacity for Z-section with Outside Angled Stiffener

Model	Finite Strip Analysis					FEM		AISI	PRO	M <sub>PRO</sub> /M <sub>AISI</sub>	M <sub>FEM</sub> /M <sub>AISI</sub>	M <sub>FEM</sub> /M <sub>PRO</sub>
	(fcr)d (MPa)	Rd	Rd(fcr)d (MPa)	(fcr)l (MPa)	fcr (MPa)	fu100 fu75	Mu100 Mu75	M <sub>AISI</sub> (N-m)	M <sub>PRO</sub> (N-m)		imp 100% imp 75% imp 25%	imp 100% imp 75% imp 25%
1	749	0.997	746	1521	746	369 352 298	737 702 594	675	673	1.00	1.09 1.04 0.88	1.09 1.04 0.88
2	645	0.976	630	1542	630	369 355 314	769 740 654	705	678	0.96	1.09 1.05 0.93	1.13 1.09 0.96
3	486	0.935	455	573	455	362 345 309	1795 1708 1532	1693	1499	0.89	1.06 1.01 0.90	1.20 1.14 1.02
4	428	0.916	392	580	392	366 339 305	1900 1764 1583	1775	1510	0.85	1.07 0.99 0.89	1.26 1.17 1.05
5	207	0.811	168	400	168	267 258 229	1991 1922 1708	2027	1635	0.81	0.98 0.95 0.84	1.22 1.18 1.04
6	204	0.808	165	393	165	275 262 232	2113 2015 1784	2131	1675	0.79	0.99 0.95 0.84	1.26 1.20 1.07
7	348	0.887	309	393	309	318 297 252	2536 2371 2008	2372	2145	0.90	1.07 1.00 0.85	1.18 1.11 0.94
8	297	0.863	256	393	256	317 292 247	2639 2435 2056	2490	2116	0.85	1.06 0.98 0.83	1.25 1.15 0.97
9	286	0.858	246	266	246	317 297 284	2779 2604 2486	2885	2239	0.78	0.96 0.90 0.86	1.24 1.16 1.11
10	269	0.849	228	266	228	318 295 278	2913 2702 2544	3020	2285	0.76	0.96 0.89 0.84	1.27 1.18 1.11
11	173	0.785	135	242	135	252 249 236	3154 3115 2947	3184	2576	0.81	0.99 0.98 0.93	1.22 1.21 1.14
12	169	0.782	132	245	132	252 190 172	3244 2448 2216	3365	2631	0.78	0.96 0.73 0.66	1.23 0.93 0.84
13	273	0.851	232	248	232	299 280 265	4032 3777 3567	3842	3327	0.87	1.05 0.98 0.93	1.21 1.14 1.07
14	235	0.829	194	248	194	294 280 254	4159 3964 3588	4052	3292	0.81	1.03 0.98 0.89	1.26 1.20 1.09
									mean	0.85	1.03	1.22
									mean		0.96	1.14
									mean		0.86	1.02

Table 3.5 Moment Capacity for Z-section with Inside Hooked Stiffener

Model	Finite Strip Analysis					FEM		AISI	PRO	M <sub>PRO</sub> /M <sub>AISI</sub>	M <sub>FEM</sub> /M <sub>AISI</sub>	M <sub>FEM</sub> /M <sub>PRO</sub>
	(fcr)d (MPa)	Rd	Rd(fcr)d (MPa)	(fcr)l (MPa)	fcr (MPa)	fu100 fu75	Mu100 Mu75	M <sub>AISI</sub> (N-m)	M <sub>PRO</sub> (N-m)		imp 100% imp 75% imp 25%	imp 100% imp 75% imp 25%
1	825	1	825	1397	825	369	738	676	676	1.00	1.09	1.09
2	866	1	866	1487	866	355	710	710	710	1.00	1.05	1.05
						298	596				0.88	0.88
						373	782				1.10	1.10
3	528	0.947	500	566	500	362	1797	1694	1539	0.91	1.06	1.17
						326	1616				0.95	1.05
						303	1503				0.89	0.98
4	545	0.952	519	569	519	366	1906	1781	1634	0.92	1.07	1.17
						335	1744				0.98	1.07
						304	1584				0.89	0.97
5	210	0.813	171	683	171	263	1964	2019	1656	0.82	0.97	1.19
						203	1513				0.75	0.91
						198	1480				0.73	0.89
6	224	0.822	184	359	184	257	1982	2121	1754	0.83	0.93	1.13
						221	1703				0.80	0.97
						208	1602				0.76	0.91
7	386	0.902	348	380	348	325	2602	2377	2229	0.94	1.09	1.17
						295	2362				0.99	1.06
						257	2058				0.87	0.92
8	407	0.909	370	386	370	325	2728	2514	2389	0.95	1.09	1.14
						292	2453				0.98	1.03
						252	2114				0.84	0.88
9	304	0.866	263	273	263	319	2799	2886	2296	0.80	0.97	1.22
10	321	0.874	281	273	273	299	2623	3027	2425	0.80	0.91	1.14
						279	2442				0.85	1.06
						325	2976				0.98	1.23
11	176	0.787	139	235	139	313	2869	3166	2615	0.83	0.95	1.18
						297	2726				0.90	1.12
						247	3090				0.98	1.18
12	183	0.793	145	238	145	191	2387	3343	2737	0.82	0.75	0.91
						189	2369				0.75	0.91
						248	3204				0.96	1.17
13	304	0.866	263	242	242	195	2522	3847	3384	0.88	0.75	0.92
						184	2371				0.71	0.87
						299	4042				1.05	1.19
14	317	0.873	277	248	248	293	3949	4095	3592	0.88	1.03	1.17
						266	3586				0.93	1.06
						298	4227				1.03	1.18
						294	4178				1.02	1.16
						261	3708				0.91	1.03
										mean	0.88	1.03
										mean	0.93	1.05
										mean	0.84	0.96

Table 3.6 Moment Capacity for Z-section with Outside Hooked Stiffener

Model	Finite Strip Analysis					FEM		AISI	PRO	M <sub>PRO</sub> /M <sub>AISI</sub>	M <sub>FEM</sub> /M <sub>AISI</sub>	M <sub>FEM</sub> /M <sub>PRO</sub>	
	(fcr)d (MPa)	Rd	Rd(fcr)d (MPa)	(fcr)l (MPa)	fcr (MPa)	fu100 fu75	Mu100 Mu75	M <sub>AISI</sub> (N-m)	M <sub>PRO</sub> (N-m)		imp 100% imp 75% imp 25%	imp 100% imp 75% imp 25%	
1	735	0.994	731	1490	731	369 355 319	738 710 637	676	671	0.99	1.09 1.05 0.94	1.10 1.06 0.95	
2	642	0.975	626	1532	626	366 345 294	768 724 617	710	683	0.96	1.08 1.02 0.87	1.12 1.06 0.90	
3	480	0.933	447	573	447	359 331 307	1780 1643 1523	1694	1496	0.88	1.05 0.97 0.90	1.19 1.10 1.02	
4	428	0.916	392	576	392	362 321 295	1888 1674 1536	1781	1518	0.85	1.06 0.94 0.86	1.24 1.10 1.01	
5	200	0.806	161	359	161	266 201 199	1982 1498 1488	2019	1623	0.80	0.98 0.74 0.74	1.22 0.92 0.92	
6	193	0.801	155	386	155	271 223 216	2089 1722 1668	2121	1652	0.78	0.98 0.81 0.79	1.26 1.04 1.01	
7	342	0.884	302	386	302	317 296 250	2535 2364 2003	2377	2137	0.90	1.07 0.99 0.84	1.19 1.11 0.94	
8	297	0.863	256	393	256	323 290 248	2711 2433 2080	2514	2134	0.85	1.08 0.97 0.83	1.27 1.14 0.97	
9	283	0.856	242	262	242	321 302 284	2814 2644 2487	2886	2241	0.78	0.97 0.92 0.86	1.26 1.18 1.11	
10	269	0.849	228	266	228	316 298 282	2894 2736 2581	3027	2300	0.76	0.96 0.90 0.85	1.26 1.19 1.12	
11	169	0.782	132	238	132	246 194 189	3082 2430 2361	3166	2573	0.81	0.97 0.77 0.75	1.20 0.94 0.92	
12	162	0.776	126	242	126	247 179 174	3191 2308 2251	3343	2607	0.78	0.95 0.69 0.67	1.22 0.89 0.86	
13	269	0.849	228	245	228	298 292 262	4023 3939 3539	3847	3323	0.86	1.05 1.02 0.92	1.21 1.19 1.06	
14	235	0.829	194	248	194	294 285 250	4169 4041 3547	4095	3315	0.81	1.02 0.99 0.87	1.26 1.22 1.07	
									mean		0.84	1.02	1.21
									mean			0.91	1.08
									mean			0.83	0.99

Table 3.7 Finite Element Results for Imperfection Magnitude 100%  
Probability of Exceedance

Model	Simple	Inside	Outside	Inside	Outside	$M_{MAX}$ (N-m)
	Lip	Angled	Angled	Hooked	Hooked	
	$M_{SL}/M_{MAX}$	$M_{IA}/M_{MAX}$	$M_{OA}/M_{MAX}$	$M_{IH}/M_{MAX}$	$M_{OH}/M_{MAX}$	
1	1	0.998	0.989	0.990	0.990	745
2	0.986	0.993	0.984	1	0.981	782
3	1	0.976	0.976	0.976	0.967	1840
4	1	0.985	0.976	0.979	0.970	1947
5	0.989	0.988	1	0.986	0.995	1991
6	1	0.894	0.897	0.841	0.887	2356
7	0.957	0.967	0.975	1	0.975	2602
8	0.913	0.942	0.967	1	0.994	2728
9	1	0.990	0.983	0.990	0.995	2828
10	1	0.993	0.972	0.993	0.965	2998
11	0.999	0.997	1	0.980	0.977	3154
12	0.995	0.938	1	0.988	0.984	3244
13	0.969	1	0.994	0.997	0.992	4056
14	0.905	1	0.976	0.992	0.978	4261

$M_{SL}$ ,  $M_{IA}$ ,  $M_{OA}$ ,  $M_{IH}$ ,  $M_{OH}$  – Z-section moment capacity for Simple Lip,  
Inside Angled, Outside Angled, Inside Hooked and Outside Hooked stiffeners  
 $M_{MAX}$  – Maximum moment capacity between  $M_{SL}$ ,  $M_{IA}$ ,  $M_{OA}$ ,  $M_{IH}$ ,  $M_{OH}$

Table 3.8 Finite Element Results for Imperfection Magnitude 75%  
Probability of Exceedance

Model	Simple	Inside	Outside	Inside	Outside	$M_{MAX}$ (N-m)
	Lip	Angled	Angled	Hooked	Hooked	
	$M_{SL}/M_{MAX}$	$M_{IA}/M_{MAX}$	$M_{OA}/M_{MAX}$	$M_{IH}/M_{MAX}$	$M_{OH}/M_{MAX}$	
1	1	0.988	0.978	0.990	0.990	718
2	1	0.997	0.978	0.985	0.957	757
3	0.975	0.964	1	0.946	0.962	1708
4	1	0.962	0.967	0.957	0.918	1823
5	0.876	0.810	1	0.788	0.780	1922
6	1	0.842	0.927	0.783	0.792	2174
7	0.967	0.995	1	0.996	0.997	2371
8	0.984	1	0.986	0.993	0.985	2470
9	1	0.954	0.958	0.965	0.973	2719
10	1	0.977	0.926	0.984	0.938	2916
11	0.888	0.870	1	0.766	0.780	3115
12	1	0.843	0.774	0.798	0.730	3160
13	0.946	0.976	0.956	1	0.998	3949
14	0.901	0.988	0.949	1	0.967	4178

Table 3.9 Finite Element Results for Imperfection Magnitude 25%  
Probability of Exceedance

Model	Simple	Inside	Outside	Inside	Outside	$M_{MAX}$ (N-m)
	Lip	Angled	Angled	Hooked	Hooked	
	$M_{SL}/M_{MAX}$	$M_{IA}/M_{MAX}$	$M_{OA}/M_{MAX}$	$M_{IH}/M_{MAX}$	$M_{OH}/M_{MAX}$	
1	0.962	0.936	0.932	0.935	1	637
2	1	0.920	0.947	0.912	0.893	691
3	1	0.978	0.988	0.969	0.982	1551
4	1	0.944	0.959	0.960	0.931	1650
5	0.966	0.899	1	0.866	0.871	1708
6	1	0.850	0.967	0.868	0.904	1845
7	0.969	0.973	0.975	1	0.973	2058
8	1	0.918	0.917	0.943	0.927	2243
9	1	0.945	0.977	0.960	0.978	2544
10	1	0.964	0.910	0.976	0.924	2794
11	0.923	0.912	1	0.804	0.801	2947
12	1	0.871	0.787	0.842	0.799	2815
13	0.936	0.982	0.995	1	0.987	3586
14	0.946	0.997	0.968	1	0.956	3708

Table 3.10 AISI and Proposed Design Approach for Flexural Members

Model	AISI Method				Proposed Design Approach for Flexural Members Method					
	Simple	Inside, Outside	Inside, Outside	$M_{MAX}$ (N-m)	Simple	Inside	Outside	Inside	Outside	$M_{MAX}$ (N-m)
	Lip	Angled	Hooked		Lip	Angled	Angled	Hooked	Hooked	
$M_{SL}/M_{MAX}$	$M_{IOA}/M_{MAX}$	$M_{IOH}/M_{MAX}$		$M_{SL}/M_{MAX}$	$M_{IA}/M_{MAX}$	$M_{OA}/M_{MAX}$	$M_{IH}/M_{MAX}$	$M_{OH}/M_{MAX}$		
1	0.991	0.998	1	676	0.991	0.998	0.996	1	0.993	676
2	0.960	0.993	1	710	0.968	0.993	0.956	1	0.962	710
3	0.996	0.999	1	1694	1	0.987	0.952	0.978	0.950	1574
4	0.974	0.997	1	1781	0.988	1	0.907	0.981	0.911	1666
5	0.959	1	0.996	2027	1	0.964	0.939	0.951	0.932	1740
6	1	0.975	0.971	2185	1	0.913	0.848	0.888	0.836	1976
7	0.921	0.998	1	2377	0.973	1	0.948	0.985	0.944	2263
8	0.843	0.991	1	2514	0.925	1	0.879	0.993	0.886	2407
9	0.997	0.999	1	2886	0.994	0.994	0.975	1	0.976	2296
10	0.975	0.998	1	3027	0.978	0.983	0.942	1	0.949	2425
11	0.946	1	0.994	3184	1	0.962	0.939	0.954	0.938	2742
12	1	0.970	0.964	3468	1	0.915	0.854	0.888	0.846	3081
13	0.901	0.999	1	3847	0.988	1	0.982	0.999	0.981	3386
14	0.811	0.990	1	4095	0.965	0.994	0.916	1	0.923	3592

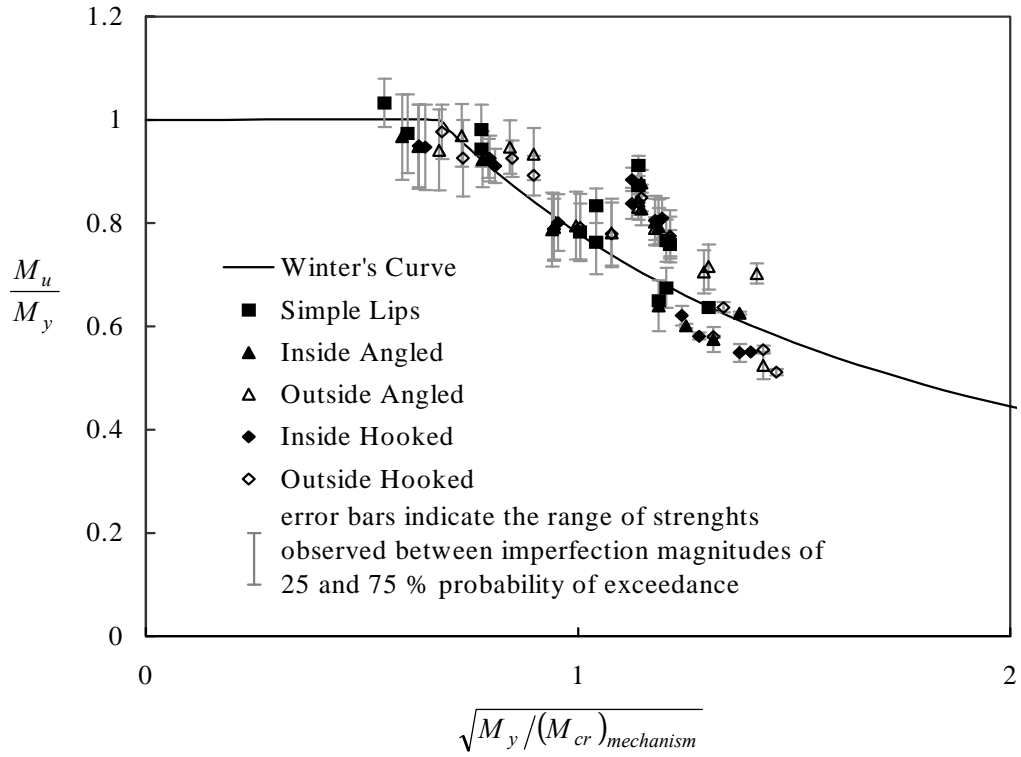


Figure 3.3 Post-Buckling Capacity by Finite Element Method

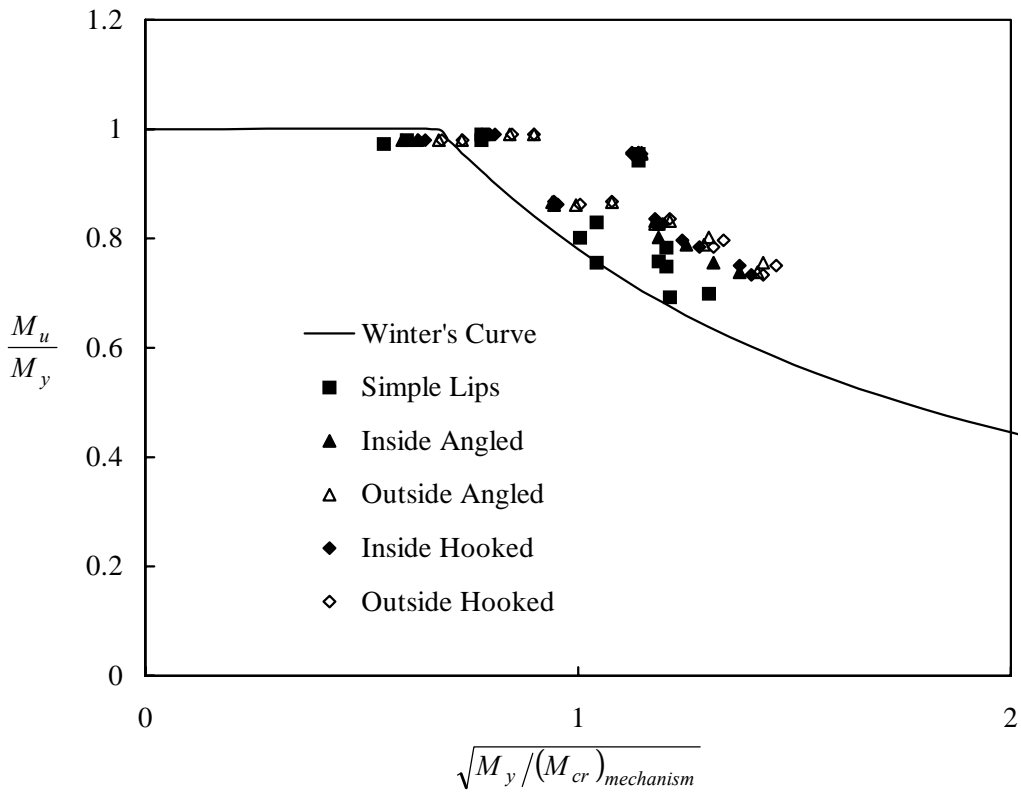


Figure 3.4 Post-Buckling Capacity by AISI Method

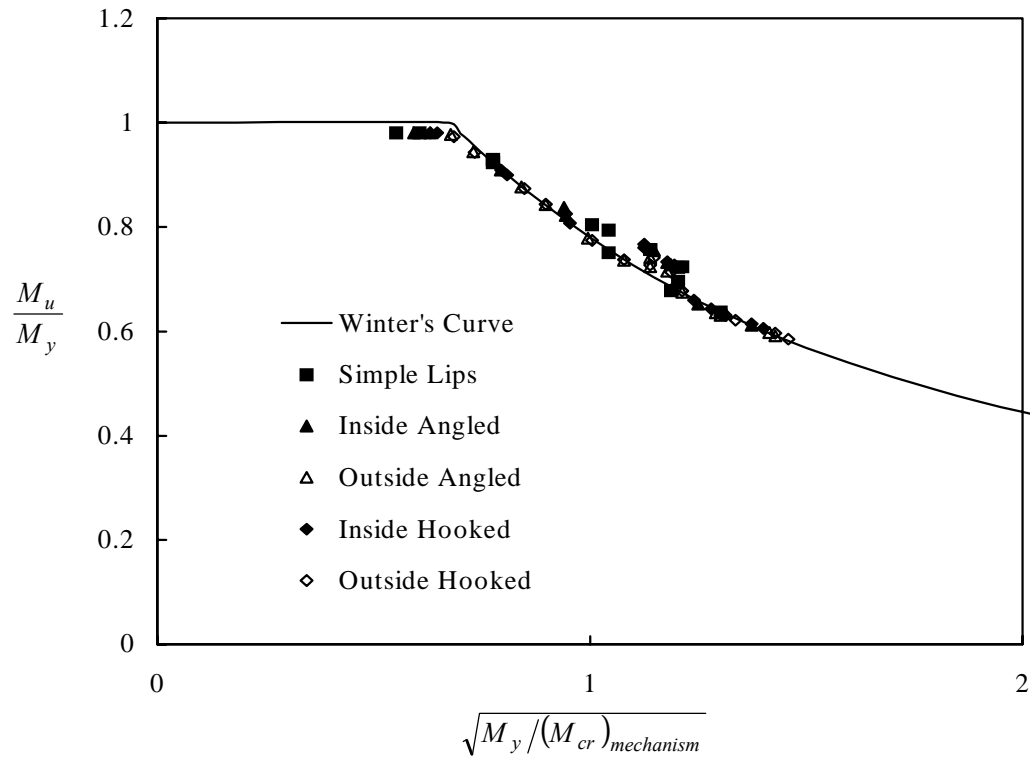


Figure 3.5 Post-Buckling Capacity by Proposed Design Approach for Flexural Members



### 3.3 Z-Section Parameter Study for Various Thickness

This second parameter study was carried out by modifying the stiffeners of a standard cross section 12ZS3.25, which has a sloping lip stiffener. The sloping lip stiffener was modified into simple lip, outside angled and inside angled stiffeners. Cross-section area for each type of stiffeners is maintained to compare the efficiency. This was done for different thickness. The finite element assumptions are the same as in the first parameter study except for the yield stress and the lateral bracing conditions. This study uses a 55 ksi yield stress and instead of fully bracing along the length as in the first parameter study, only four brace points are used. Brace points one at each end and additional ones at one-third of the length, which is the same length as the half-wave lengths of the distortional mode that gives the least buckling strength. Figure 3.7 shows the second parameter study boundary and loading conditions. For this cross section and bracing length, full formation of the distortional mode is still possible without causing a lateral-torsional failure. Figure 3.6 and Table 3.11 summarize the geometry of the members. Results for each type of stiffeners for this parameter study are shown in Table 3.12. Table 3.13, 3.14, 3.15, 3.16 and Figure 3.8, 3.9, 3.10 summarizes the results for different approaches.

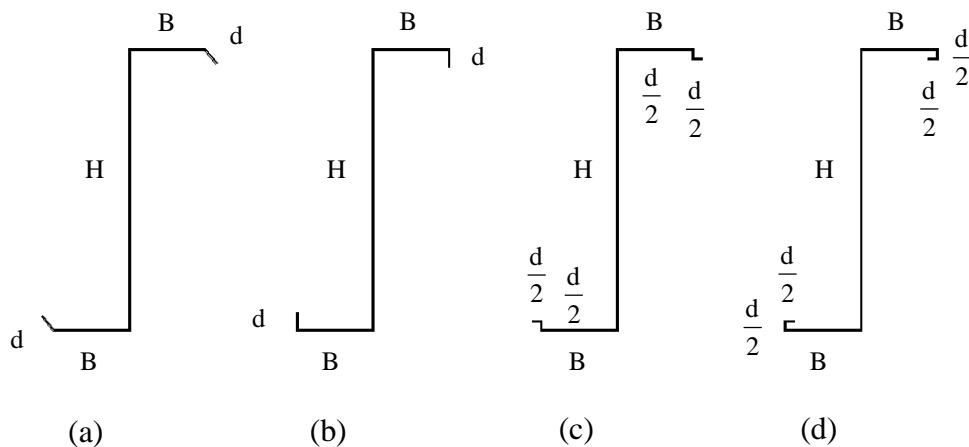


Figure 3.6 Z-section 12ZS3.25 with (a) Sloping Lip Stiffener 50 degree respect to the flange (b) Simple Lip Stiffener (c) Outside Angled Stiffener (d) Inside Angled Stiffener

Table 3.11 Summary of Models Geometry

Model	Dimensions							
	H (in.)	B (in.)	d (in.)	t (in.)	Length, L (in.)			
					Sloping Lip	Simple Lip	Inside Angled	Outside Angled
1	12	3.25	0.75	0.135	54	60	51	51
2	12	3.25	0.75	0.105	63	69	57	60
3	12	3.25	0.75	0.090	69	75	63	75
4	12	3.25	0.75	0.075	75	84	69	72
5	12	3.25	0.75	0.060	84	96	78	81

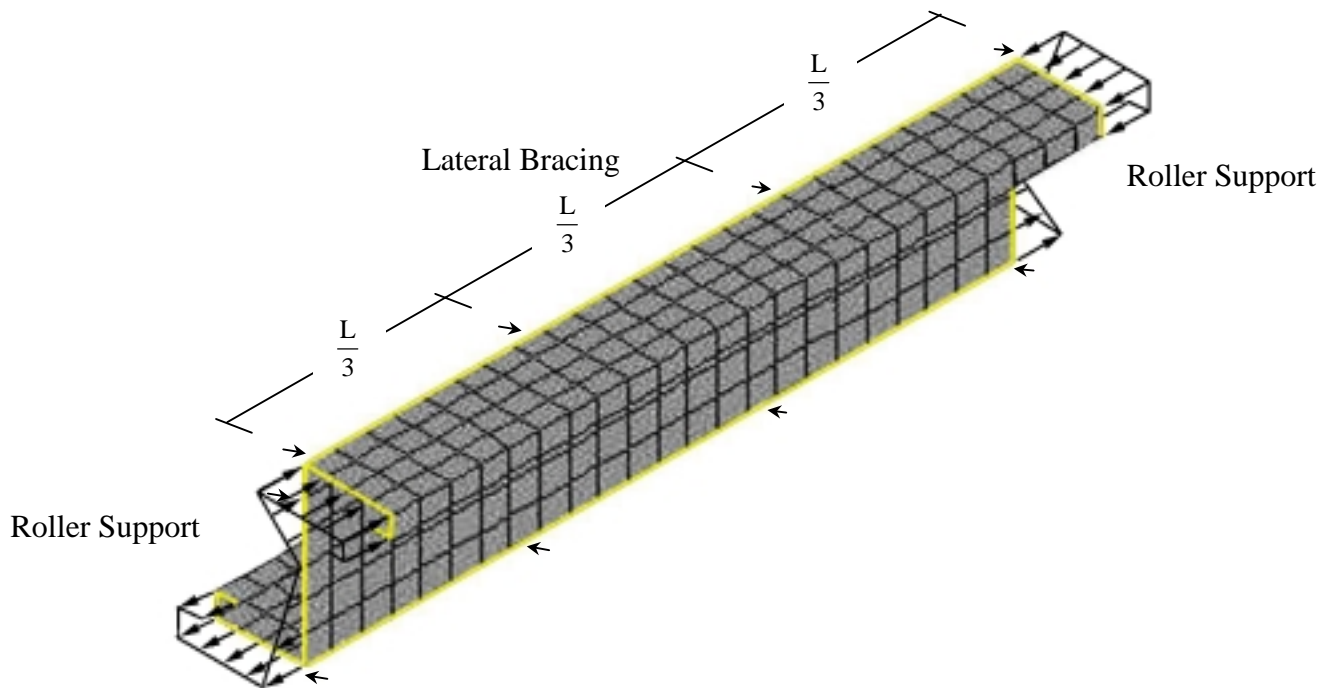


Figure 3.7 Boundary and Loading Condition

Table 3.12 Moment Capacity for Z-section 12ZS3.25 with Sloping Lip, Simple Lip, Outside Angled, Inside Angled Stiffeners

Model	Finite Strip Analysis					FEM		AISI	PRO	M <sub>PRO</sub> /M <sub>AISI</sub>	M <sub>FEM</sub> /M <sub>AISI</sub>	M <sub>FEM</sub> /M <sub>PRO</sub>	
	(fcr)d (ksi)	Rd	Rd(fcr)d (ksi)	(fcr)l (ksi)	fcr (ksi)	fu100 fu75 fu25	Mu100 Mu75 Mu25	M <sub>AISI</sub> (kip-in.)	M <sub>PRO</sub> (kip-in.)		imp 100% imp 75% imp 25%	imp 100% imp 75% imp 25%	
Sloping Lip Stiffener	1	56	0.888	50	77	50	51 48 41	468 436 378	463	397	0.86	1.01 0.94 0.82	1.18 1.10 0.95
	2	40	0.838	33	59	33	45 36 37	322 262 267	327	275	0.84	0.98 0.80 0.82	1.17 0.95 0.97
	3	33	0.809	26	46	26	41 31 30	253 194 185	263	219	0.83	0.96 0.74 0.70	1.15 0.89 0.84
	4	26	0.776	20	32	20	36 27 26	188 141 135	192	167	0.87	0.98 0.73 0.70	1.13 0.84 0.81
	5	20	0.738	15	21	15	28 23 22	118 94 91	134	119	0.89	0.88 0.71 0.68	0.99 0.79 0.76
Simple Lip Stiffener	1	71	0.922	65	102	65	54 48 45	488 433 411	473	426	0.90	1.03 0.92 0.87	1.14 1.02 0.96
	2	51	0.875	45	63	45	48 40 37	338 283 264	339	302	0.89	1.00 0.83 0.78	1.12 0.94 0.88
	3	43	0.847	36	46	36	44 36 35	268 221 214	276	242	0.88	0.97 0.80 0.78	1.11 0.91 0.88
	4	34	0.816	28	32	28	35 33 34	180 170 174	204	186	0.91	0.88 0.83 0.86	0.96 0.91 0.94
	5	26	0.779	21	21	21	29 29 28	120 122 118	145	135	0.93	0.83 0.84 0.81	0.89 0.90 0.87
Outside Angled Stiffener	1	54	0.881	47	69	47	50 46 41	454 420 376	480	393	0.82	0.95 0.88 0.78	1.16 1.07 0.96
	2	38	0.830	31	54	31	43 36 37	305 257 264	345	271	0.79	0.89 0.74 0.77	1.13 0.95 0.97
	3	31	0.800	24	46	24	39 31 31	239 192 188	286	215	0.75	0.84 0.67 0.66	1.11 0.89 0.87
	4	24	0.766	19	32	19	34 27 26	176 141 135	208	163	0.78	0.84 0.67 0.65	1.08 0.86 0.82
	5	18	0.728	13	21	13	27 23 23	114 97 95	144	116	0.81	0.79 0.68 0.66	0.98 0.84 0.81

Inside Angled Stiffener	1	59	0.896	53	77	53	51	466	480	406	0.85	0.97	1.15	
							46	420				0.88	1.04	
	2	42	0.846	36	58	36	41	368	345	283	0.82	0.77	0.91	
							38	269				0.92	1.13	
							38	270				0.78	0.95	
	3	34	0.817	28	45	28	41	255	286	244	0.85	0.78	0.96	
							35	213				0.89	1.04	
							34	207				0.74	0.87	
	4	27	0.784	22	31	22	35	182	208	172	0.83	0.72	0.85	
							29	149				0.87	1.06	
							28	145				0.72	0.87	
	5	21	0.746	16	20	16	28	117	144	123	0.85	0.70	0.84	
							27	112				0.82	0.95	
							24	100				0.78	0.91	
												0.69	0.81	
											mean	0.85	0.93	1.09
											mean		0.79	0.93
											mean		0.76	0.89

Table 3.13 Finite Element Results for Imperfection Magnitude 100%  
Probability of Exceedance

Model	Sloping Lip	Simple Lip	Outside Angled	Inside Angled	$M_{MAX}$ (kip-in.)
	$M_{SLL}/M_{MAX}$	$M_{SL}/M_{MAX}$	$M_{IA}/M_{MAX}$	$M_{OA}/M_{MAX}$	
1	0.960	1	0.930	0.955	488
2	0.950	1	0.902	0.941	338
3	0.941	1	0.892	0.949	268
4	1	0.954	0.935	0.970	188
5	0.983	1	0.946	0.977	120

$M_{SLL}$ ,  $M_{SL}$ ,  $M_{IA}$ ,  $M_{OA}$  – Z-section moment capacity for Sloping Lip, Simple Lip,  
Inside Angled, Outside Angled stiffeners

$M_{MAX}$  – Maximum moment capacity between  $M_{SLL}$ ,  $M_{SL}$ ,  $M_{IA}$ ,  $M_{OA}$

Table 3.14 Finite Element Results for Imperfection Magnitude 75%  
Probability of Exceedance

Model	Sloping	Simple	Outside	Inside	$M_{MAX}$ (kip-in.)
	Lip	Lip	Angled	Angled	
	$M_{SLL}/M_{MAX}$	$M_{SL}/M_{MAX}$	$M_{IA}/M_{MAX}$	$M_{OA}/M_{MAX}$	
1	1	0.995	0.965	0.965	436
2	0.925	1	0.908	0.952	283
3	0.878	1	0.867	0.962	221
4	0.828	1	0.828	0.880	170
5	0.774	1	0.801	0.919	122

Table 3.15 Finite Element Results for Imperfection Magnitude 25%  
Probability of Exceedance

Model	Sloping	Simple	Outside	Inside	$M_{MAX}$ (kip-in.)
	Lip	Lip	Angled	Angled	
	$M_{SLL}/M_{MAX}$	$M_{SL}/M_{MAX}$	$M_{IA}/M_{MAX}$	$M_{OA}/M_{MAX}$	
1	0.920	1	0.915	0.895	411
2	0.987	0.976	0.975	1	270
3	0.864	1	0.878	0.965	214
4	0.772	1	0.772	0.831	174
5	0.770	1	0.805	0.850	118

Table 3.16 AISI and Proposed Design Approach for Flexural Members

Model	AISI Method				Proposed Design Approach for Flexural Members Method				
	Sloping	Simple	Outside, Inside	$M_{MAX}$ (kip-in.)	Sloping	Simple	Outside	Inside	$M_{MAX}$ (kip-in.)
	Lip	Lip	Angled		Lip	Lip	Angled	Angled	
$M_{SLL}/M_{MAX}$	$M_{SL}/M_{MAX}$	$M_{IOA}/M_{MAX}$		$M_{SLL}/M_{MAX}$	$M_{SL}/M_{MAX}$	$M_{IA}/M_{MAX}$	$M_{OA}/M_{MAX}$		
1	0.964	0.986	1	480	0.931	1	0.921	0.951	426
2	0.948	0.983	1	345	0.913	1	0.899	0.938	302
3	0.919	0.963	1	286	0.898	0.993	0.882	1	244
4	0.921	0.978	1	208	0.895	1	0.876	0.923	186
5	0.924	1	0.996	145	0.885	1	0.863	0.914	135

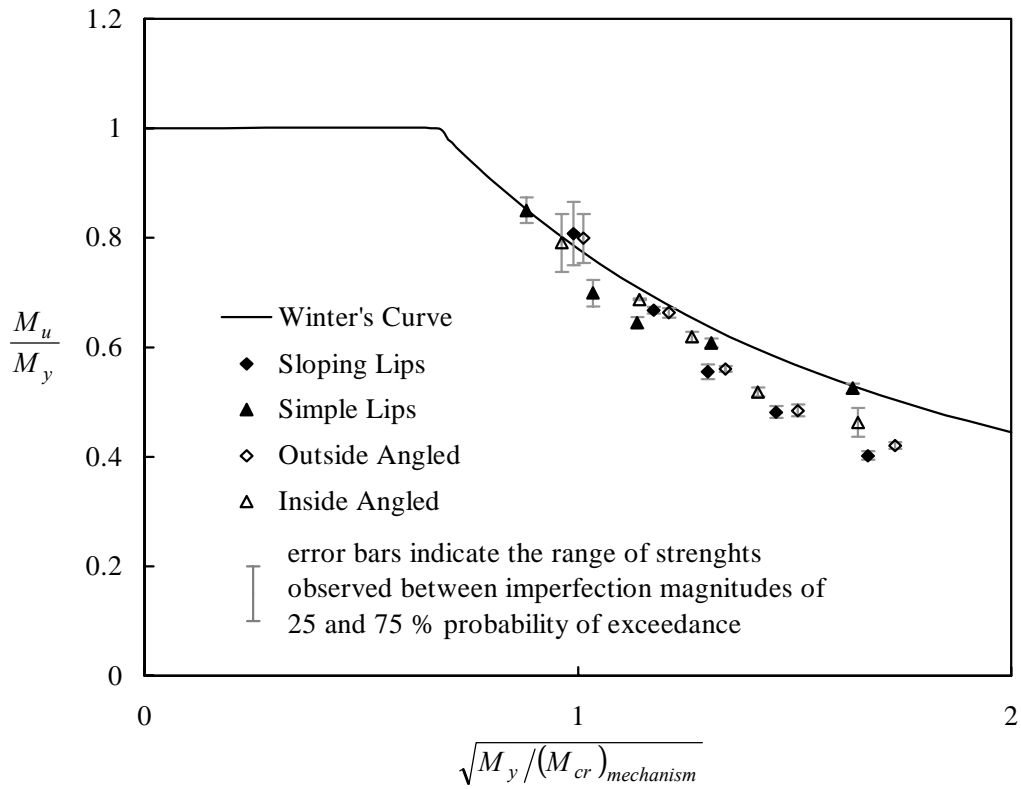


Figure 3.8 Post-Buckling Capacity by Finite Element Method

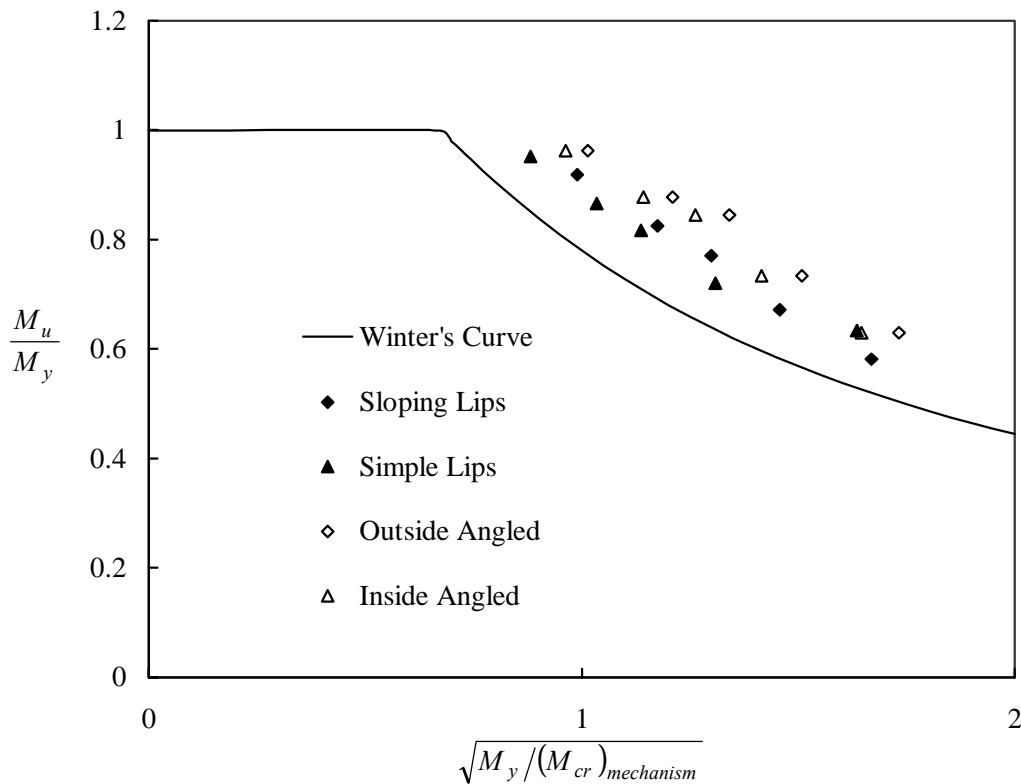


Figure 3.9 Post-Buckling Capacity by AISI Method

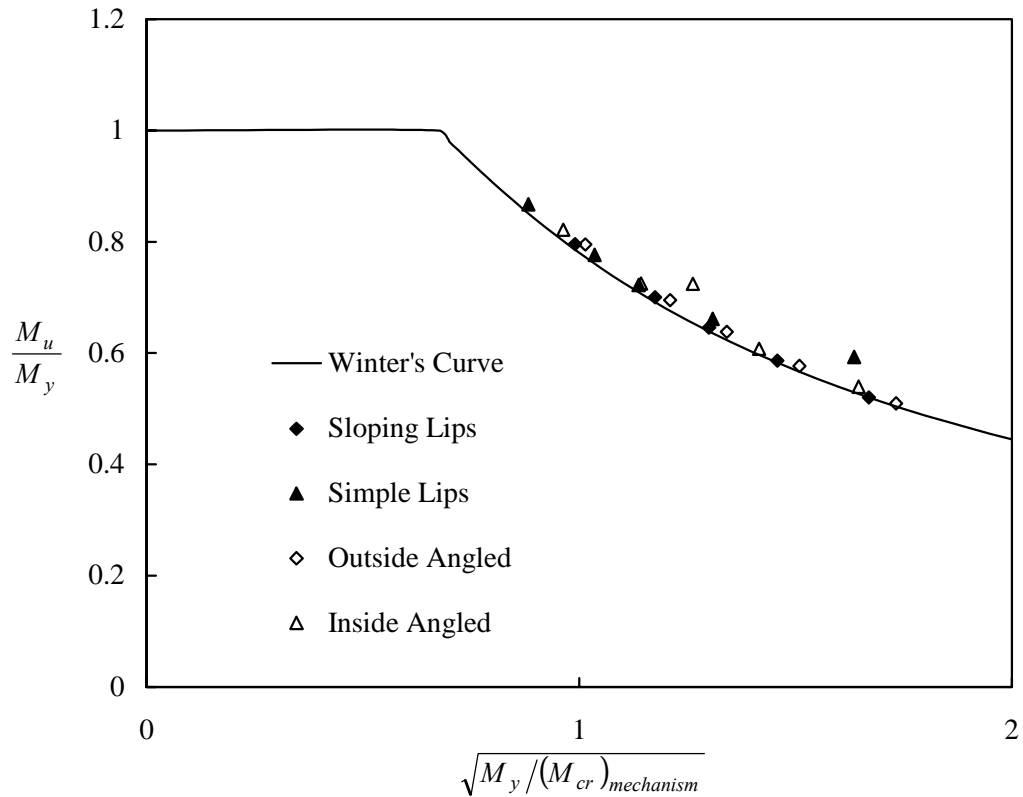


Figure 3.10 Post-Buckling Capacity by Proposed Design Approach for Flexural Members

### 3.4 Conclusions and Verification of the Proposed Design Approach

The first Z-section Parameter study of the simple lip, inside angled, outside angled, inside hooked and outside hooked stiffeners reveals that the AISI (1996) approach gives moment capacity closer to the perfect FEM model. While the proposed design approach for flexural members given by Schafer (1997), Schafer and Peköz (1999) gives a more conservative result closer to the imperfection magnitude of 25% probability of exceedance which these thin wall structure are likely to have. Almost all cases of the FEM analysis for the second parameter study suggest that for this 12ZS3.25 cross section the simple lip stiffeners give the highest moment capacity. While the AISI and the proposed design approach for flexural members given by Schafer (1997), Schafer and Peköz (1999) suggests outside angled and inside angled instead. There are no physical test results for these kinds of complex stiffeners. Some test results of members with cross sections such as in Figure 3.1 are desired to verify this approach.

#### 4 Verification of the Reduction Factor for Distortional Buckling, $R_d$

Schafer (1997), Schafer and Peköz (1999) developed the expression for  $R_d$  based on post-buckling capacity as shown in Figure 1.2, 1.3 and from the experimental results of Hancock et al. (1994). Based on the FEM results in the previous study section 3.1, two alternative  $R_d$  expressions are developed here  $(R_d)_a$  and  $(R_d)_b$ . The alternative expressions are then compared with the original  $R_d$ .

By combining the FEM results of different types of stiffeners from Figure 3.3 a regression line may be obtained as in Figure 4.1. A new expression for  $(R_d)_a$  can be found by letting  $R_d$  be a variable and recalculating back to find the best  $(R_d)_a$  that fits the FEM regression line from the proposed design approach for flexural members given by Schafer (1997), Schafer and Peköz (1999). The result of this attempt is shown in Figure 4.2. As seen the data points do not all match the regression line due to the condition that  $(R_d)_a$  is to be only equal to or less than one. These new  $R_d$  values result from all models which are  $(R_d)_a$  - data in Figure 4.3 are the data points of the new expression  $(R_d)_a$ . The same approach is also used to develop  $(R_d)_b$  but instead of trying to match the FEM regression line as  $(R_d)_a$  the expression for  $(R_d)_b$  is found based on trying to match each model to its own FEM result. Results of this are shown in Figure 4.5 and 4.6.

The original  $R_d$  is also plotted in Figure 4.3 and 4.6 to compare with  $(R_d)_a$  and  $(R_d)_b$ . As seen  $(R_d)_a$  is very close to  $R_d$  but  $(R_d)_b$  gives a larger reduction factor than  $R_d$ . Figures 4.4 and 4.7 show the results from using the two new expressions in the proposed design approach for flexural members given by Schafer (1997), Schafer and Peköz (1999). Comparing these figures with Figure 3.5 one can see that they do not differ much, thus verifying that the original  $R_d$  used in the previous study is suitable for the overall cross sections in this study.



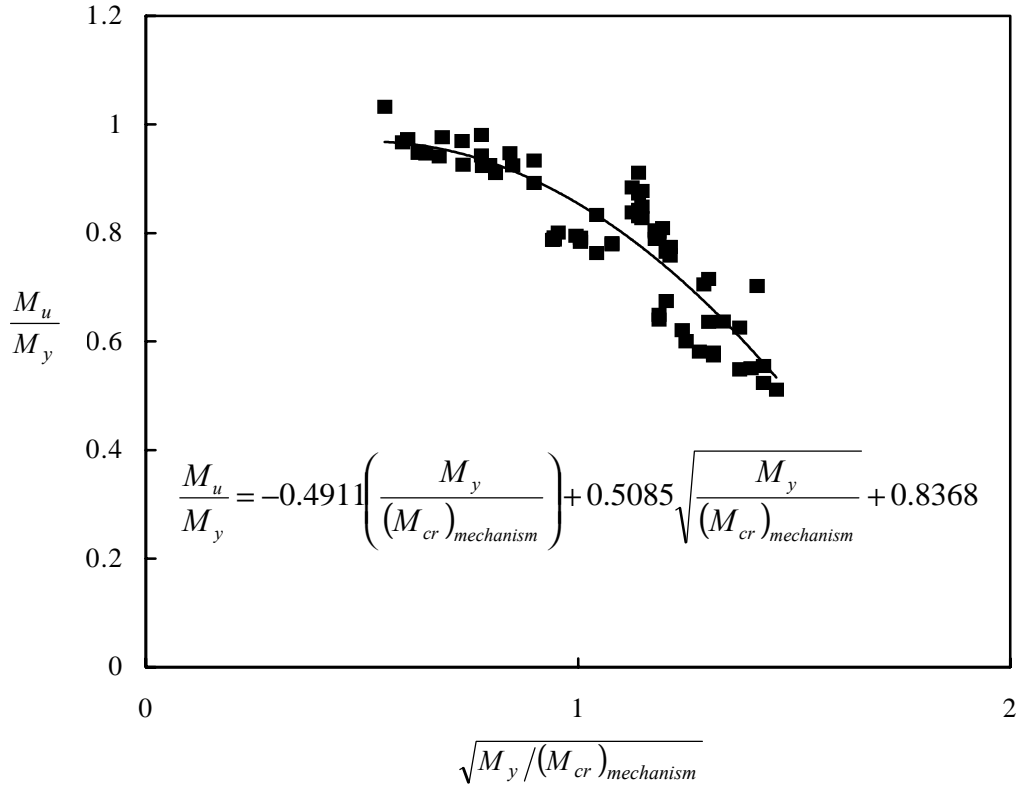


Figure 4.1 Post-Buckling Capacity by Finite Element Method (same as figure 3.3)

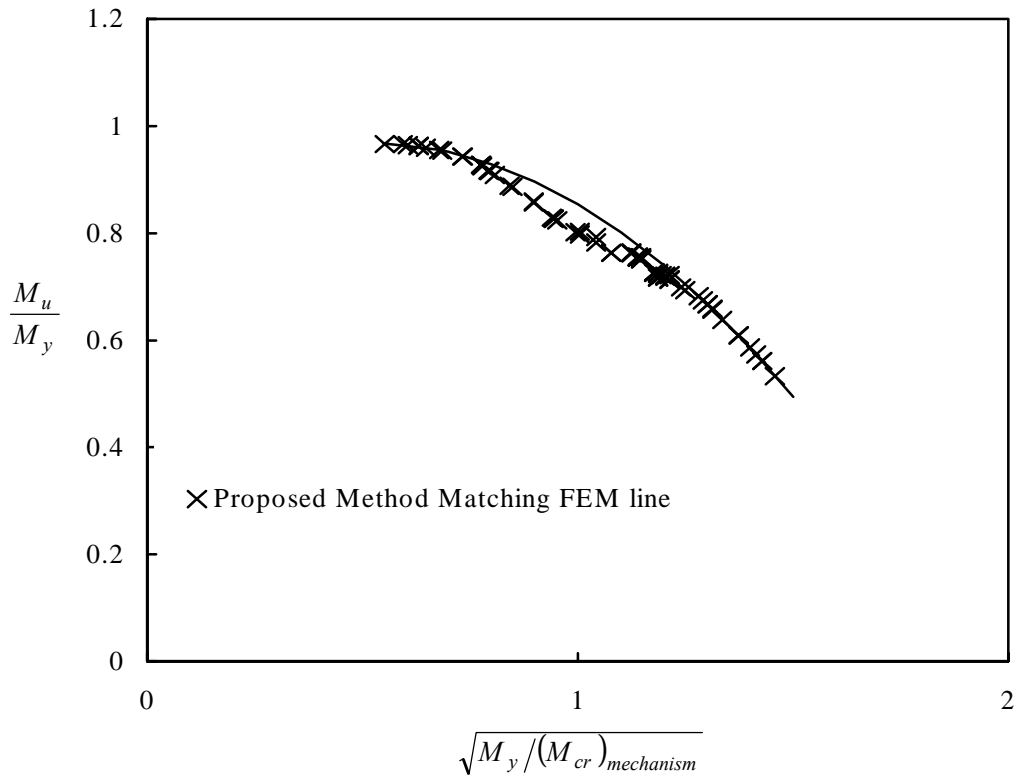


Figure 4.2 Post-Buckling Capacity by Proposed Method with  $(R_d)_a$  - data

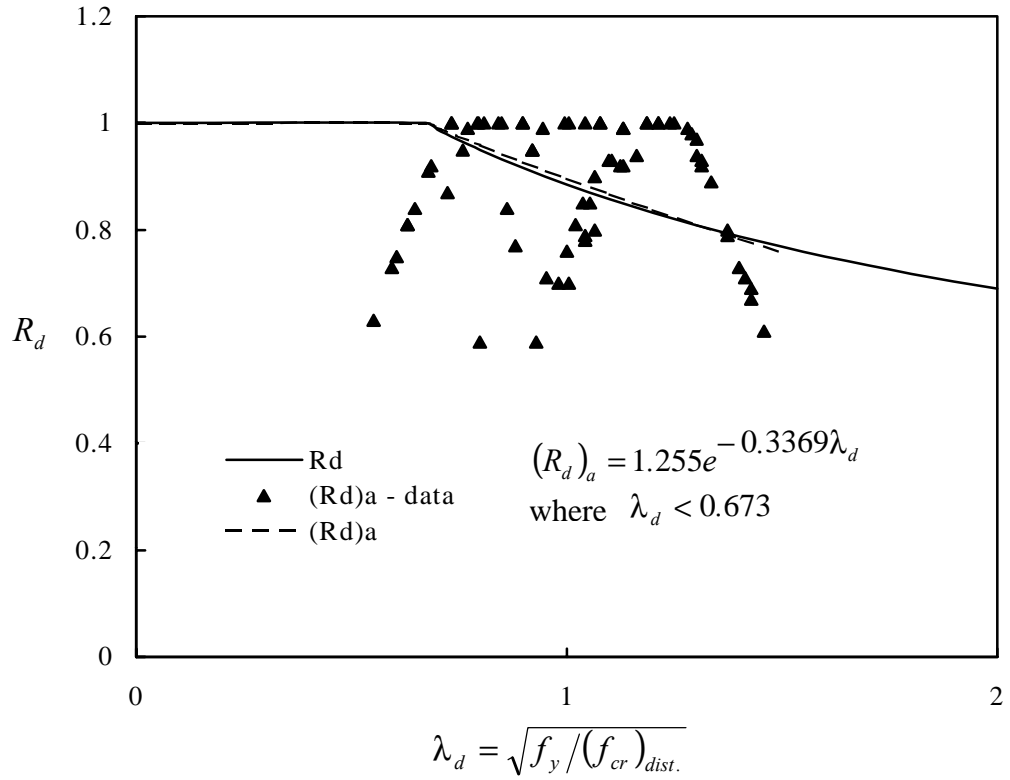


Figure 4.3 Reduction Factor for Distortional Buckling  $R_d$  vs.  $(R_d)_a$

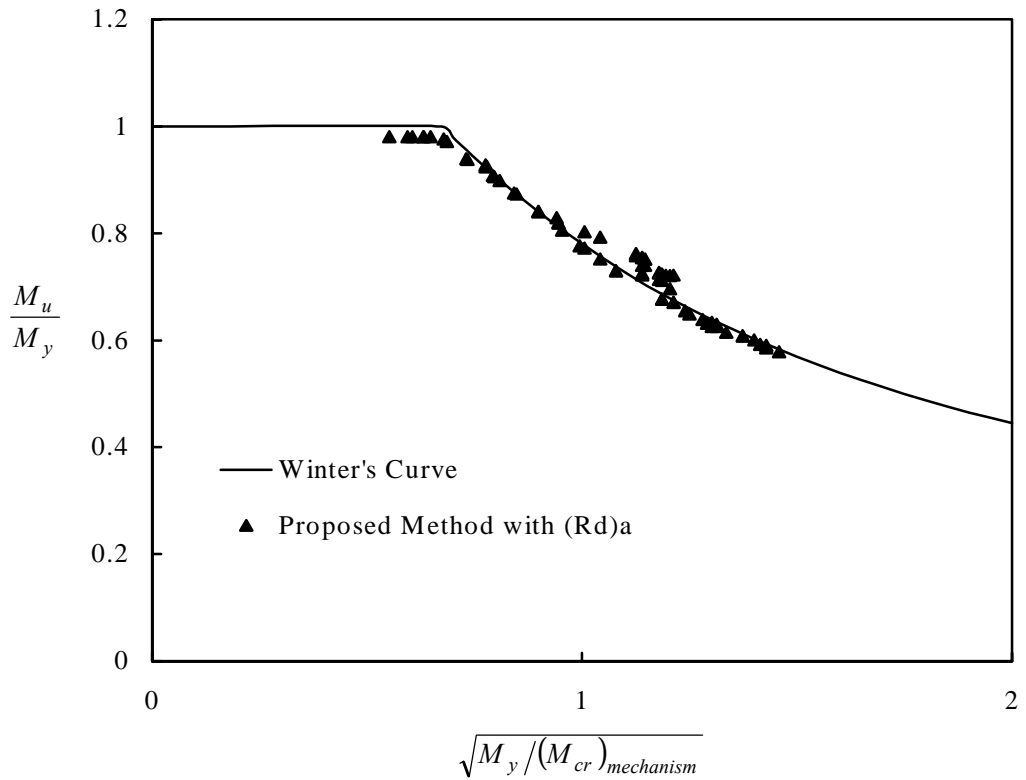


Figure 4.4 Post-Buckling Capacity by Proposed Method with  $(R_d)_a$

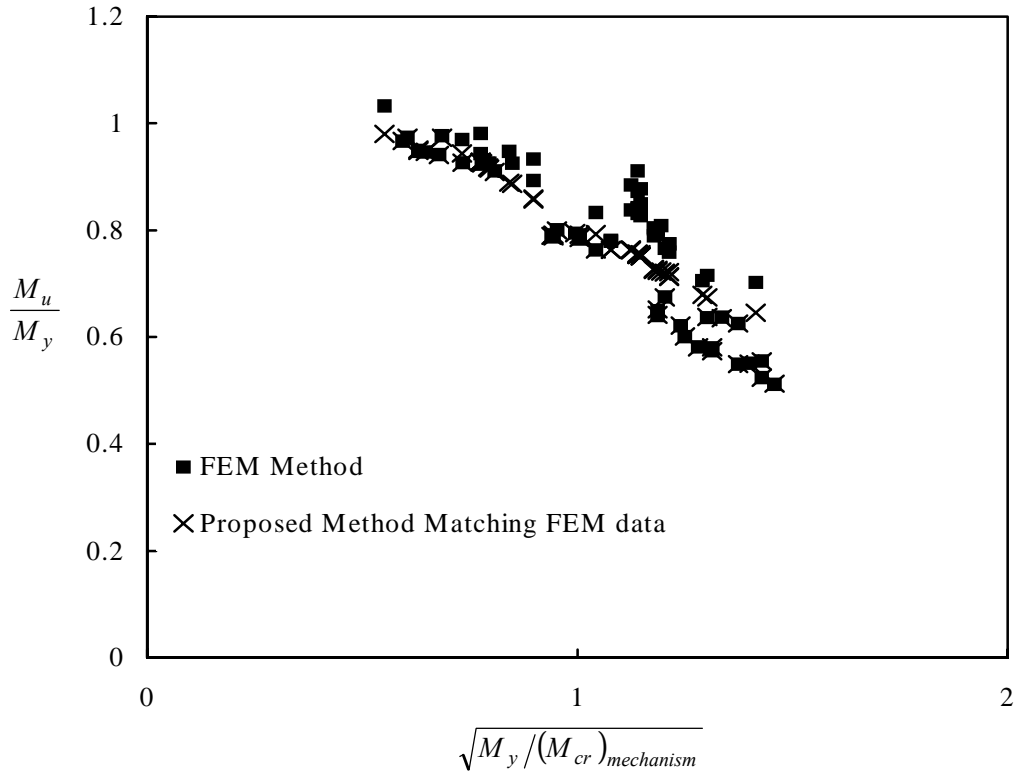


Figure 4.5 Post-Buckling Capacity by Proposed Method with  $(R_d)_b$  - data

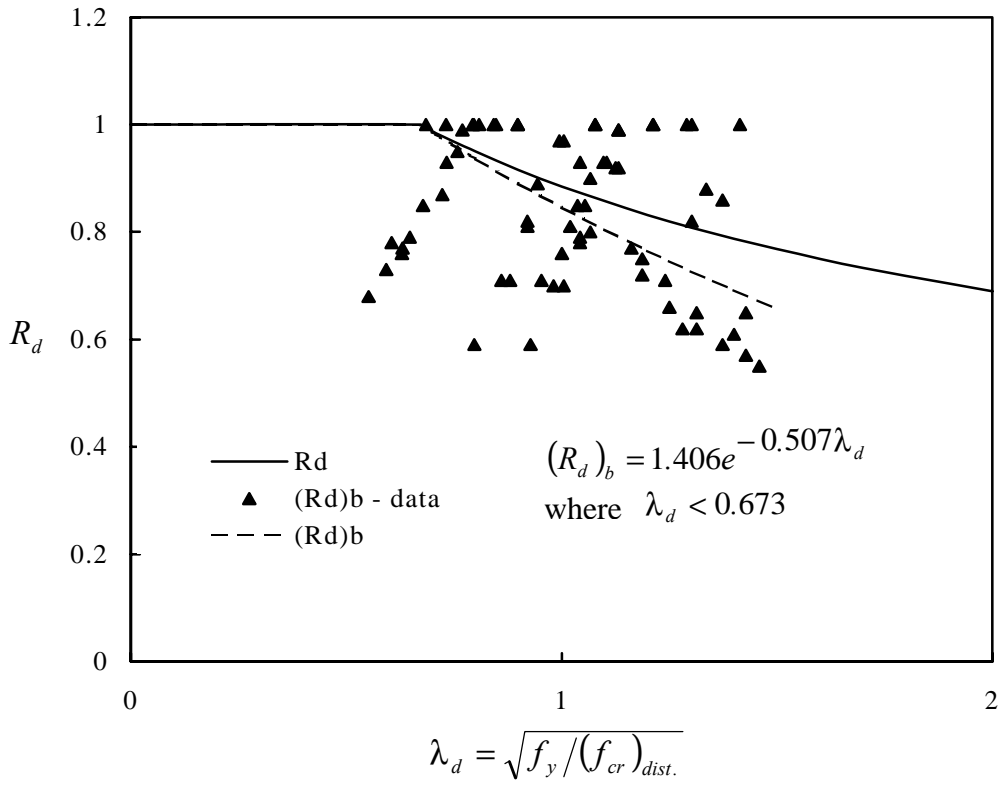


Figure 4.6 Reduction Factor for Distortional Buckling  $R_d$  vs.  $(R_d)_b$

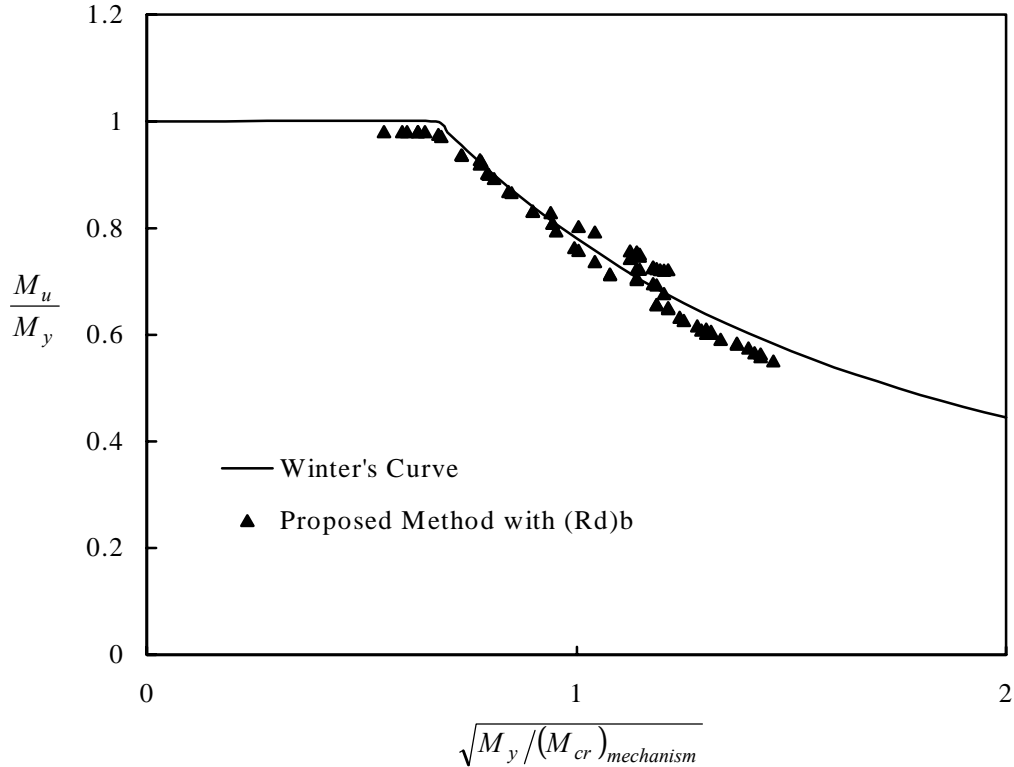


Figure 4.7 Post-Buckling Capacity by Proposed Method with  $(R_d)_b$

## 5 Cross Section Optimization Study I

A computer program CU-EWA was developed to compute the nominal flexural strength,  $M_n$ , and the nominal axial strength,  $P_n$ , using two approaches. These approaches are the AISI (1996) method of initial yielding and the proposed design approach for flexural members given by Schafer (1997), Schafer and Peköz (1999). This program, CU-EWA, is used here to perform parameter studies on the cross section optimization. The nominal flexural strength divided by the cross section area is used here as the cross section efficiency index where three parameters are investigated independently: flange width, stiffener length and stiffener/flange ratio. For each parameter study four different types of stiffeners are considered to compare their advantages. Results from two approaches, the AISI (1996) initial yielding method and the proposed design approach for flexural members given by Schafer (1997), Schafer and Peköz (1999) are also compared. For the proposed method the local and distortional buckling stresses are required. Therefore, a finite strip analysis, CUFSM, was performed for each cross section first. This parameter study was carried out for a standard cross section 12ZS3.25x090 with modified lips as in Figure 3.6.

### 5.1 Parameter Study I – Flanges Width Optimization

For the four types of stiffeners the flange width is varied to find the cross section that gives the highest efficiency index,  $M_n/A$ . Figure 5.1 shows how the geometry changes as the flange width increases and Figure 5.2 shows the results from this parameter study. The vertical line at flange width 3.25 in. is for the original cross section dimensions. Results from the proposed method in this parameter study suggest that by changing the stiffeners to simple lip and decreasing the flange width from the original values the cross section efficiency may be increased.

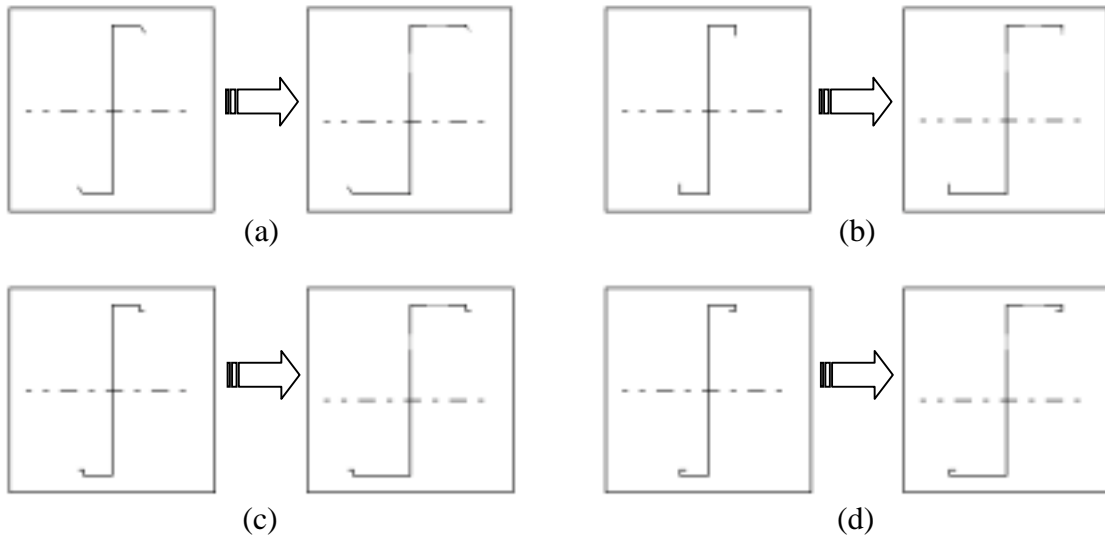


Figure 5.1 12ZS3.25x090 with (a) Sloping Lip Stiffener 50 degree respect to the flange (b) Simple Lip Stiffener (c) Outside Angled Stiffener (d) Inside Angled Stiffener

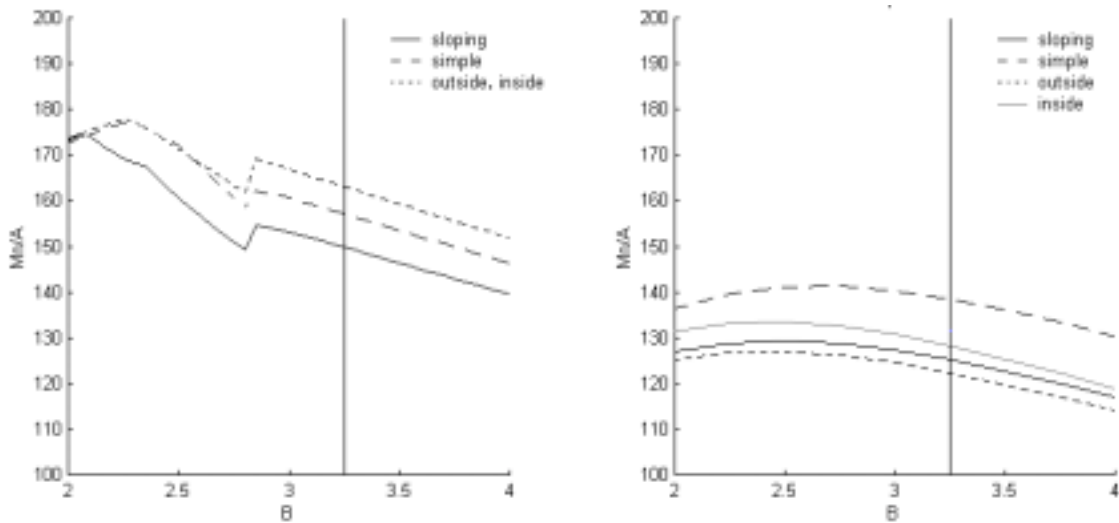


Figure 5.2 Flanges Width Optimization (a) AISI Method (b) Proposed Method

## 5.2 Parameter Study II – Stiffeners Length Optimization

Considering the stiffener length to be a varied variable and all others dimensions to be constant an optimum stiffener length may be found for different types of stiffeners. Figure 5.3 shows how the geometry changes as the stiffener length increases and Figure 5.4 shows the results from this parameter study. The vertical line at stiffener length 0.75 in. is the original cross section value. Results from the proposed method in this parameter study suggest that a simple lip with a length of 1 in. is the best section considering cross section efficiency.

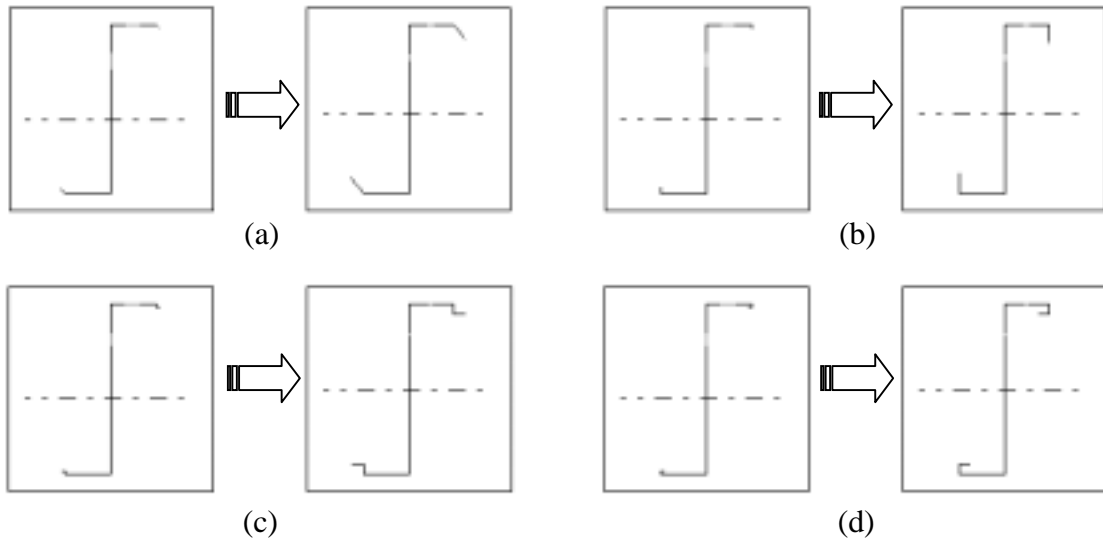


Figure 5.3 12ZS3.25x090 with (a) Sloping Lip Stiffener 50 degree respect to the flange (b) Simple Lip Stiffener (c) Outside Angled Stiffener (d) Inside Angled Stiffener

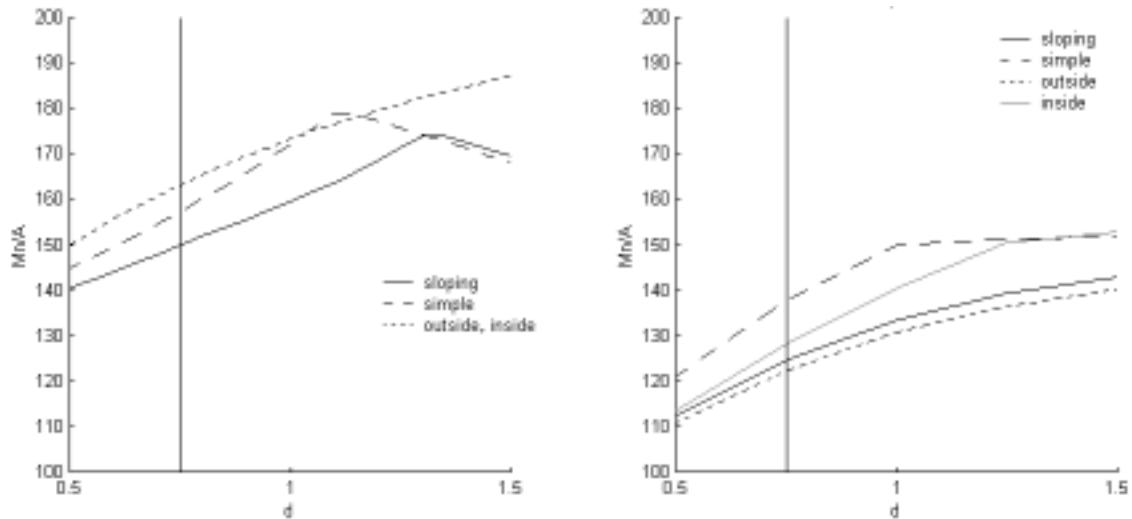


Figure 5.4 Stiffeners Length Optimization (a) AISI Method (b) Proposed Method

### 5.3 Parameter Study III – Stiffener/Flange Ratio Optimization

By changing both the stiffener and flange lengths but still maintaining the cross section area, the best stiffener/flange ratio may be found. Figure 5.5 shows how the geometry changes as the stiffener/flange ratio increases and Figure 5.6 shows the results from this parameter study. The first stiffener/flange ratio, 0.23, is the original cross section value. Results from the proposed method suggest that a simple lip with a stiffener/flange ratio of 0.33 is the best section considering efficiency.

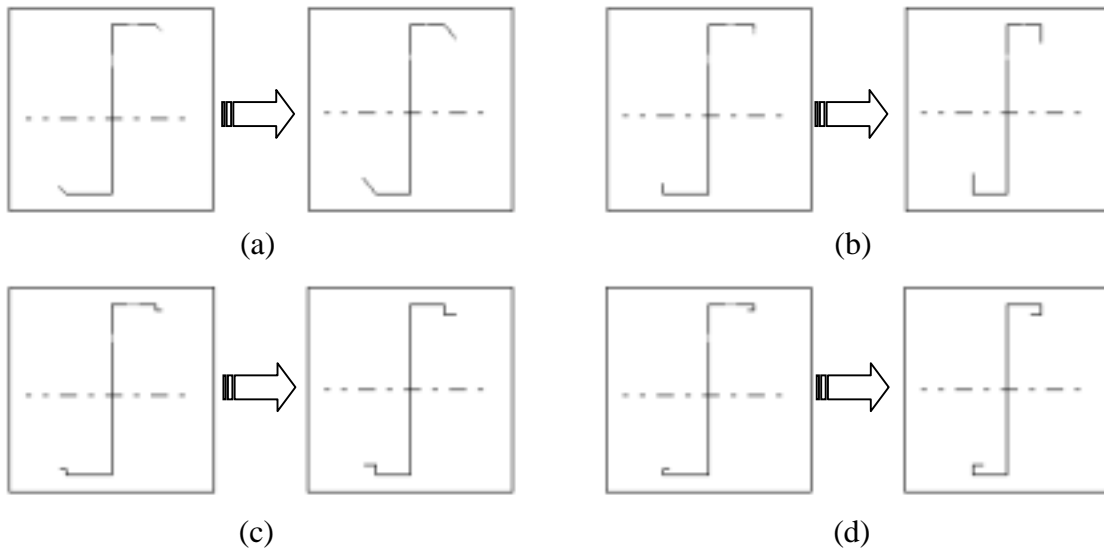


Figure 5.5 12ZS3.25x090 with (a) Sloping Lip Stiffener 50 degree respect to the flange (b) Simple Lip Stiffener (c) Outside Angled Stiffener (d) Inside Angled Stiffener

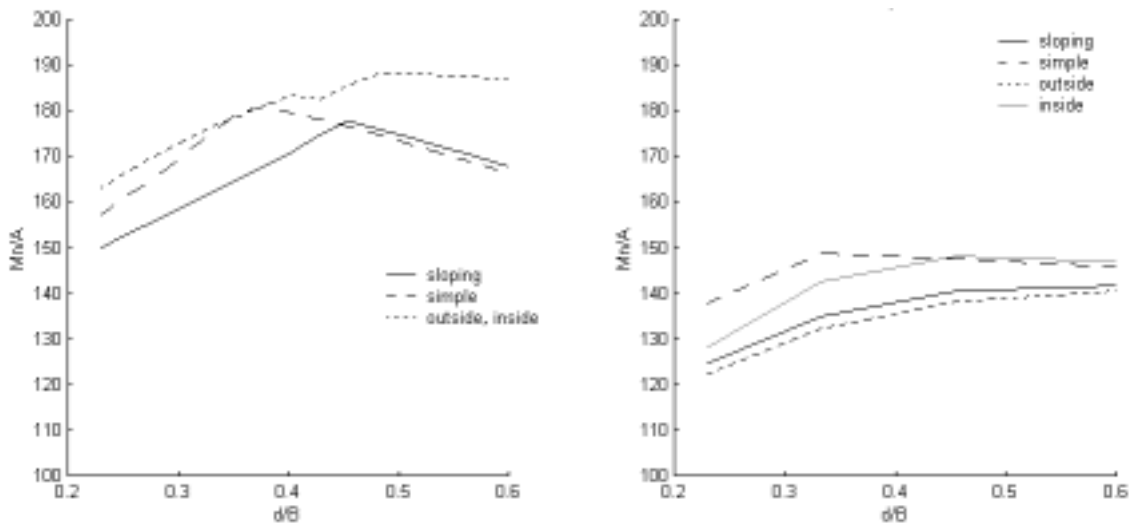


Figure 5.6 Stiffener/Flange Ratio Optimization (a) AISI Method (b) Proposed Method

## 6 Cross Section Optimization Study II

This second cross section optimization study was carried out by modifying the stiffeners of a standard cross section 9CS3x060, which has a simple lip stiffener. The simple lip stiffener was modified into different types of inside angled and outside angled stiffeners as shown in Figure 6.1. The nominal flexural strength divided by the cross section area is used here as the cross section efficiency index where the total length of the stiffener,  $d$  is increased to find the optimal section. Three different approaches to calculate the section capacity, the FEM, AISI (1996) initial yielding method and the proposed design approach for flexural members given by Schafer (1997), Schafer and Peköz (1999) are compared. Results from these different approaches are shown in Figure 6.3, 6.4, 6.5, 6.6 and 6.7.

### 6.1 Finite Element Modeling Assumptions

In this study the length of the member is taken as the half-wave lengths of the distortional mode that gives the least buckling strength. Lateral bracings are only provided by roller supports at the ends. For this cross section and member length, full formation of the distortional mode is still possible without causing a lateral-torsional failure. To avoid localized failure at the ends, the constant moment modeled by nodal loads is distributed into the first two rows of elements. Boundary and loading conditions are shown in Figure 6.2. The material model used is elastic-plastic with strain hardening and  $f_y = 65$  ksi. Residual stress throughout the thickness in the longitudinal direction is assumed to be 25% of the yield stress in the flange, 40% of the yield stress in the web and 30% at of the yield stress in the corners. The residual stresses are also assumed to be tension on the outside and compression in the inside of the section. Initial geometric imperfections are introduced by superimposing the eigenmodes for the local and distortional buckling. The imperfection magnitude 50% probability of exceedance based on the statistical summary provided in Schafer (1997), Schafer and Peköz (1999) is used.

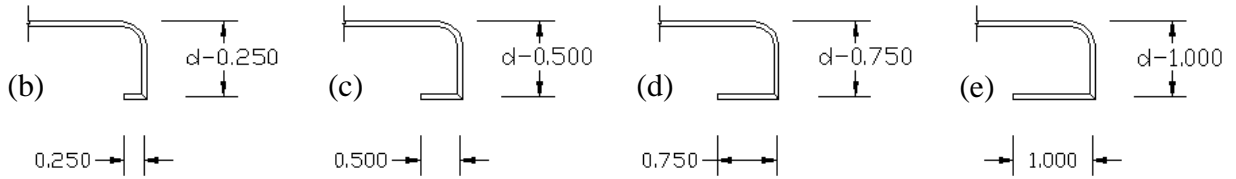


## 6.2 Conclusion

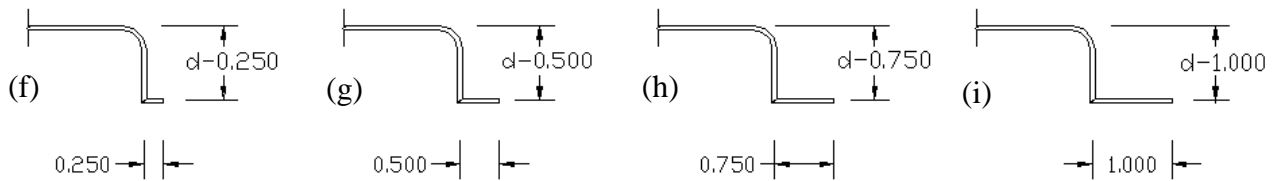
All three approaches agree well for the simple lip case that an optimized section may be obtained by increasing the stiffener length from the original given length of  $d = 0.5$  in. to about  $d = 1$  in. The current AISI (1996) method does not distinguish between the inside and outside angle stiffener. Results also show that this method is unconservative compared with the other two approaches. The overall trend of the results between the FEM and the proposed method shows agreement; that is, it is more efficient to have inside angled stiffeners than simple lip or outside angled stiffeners. A wider range of scattered data is obtained from the FEM due to the initial geometric imperfection. A study on sections with higher material yield stress has also been considered. Results are similar to those obtained here where the inside angled stiffeners has slight advantage over the simple lip and outside angled stiffeners



Simple Lip Stiffener



Inside Angled Stiffener



Outside Angled Stiffener

Figure 6.1 Various Types of Stiffener Under Consideration

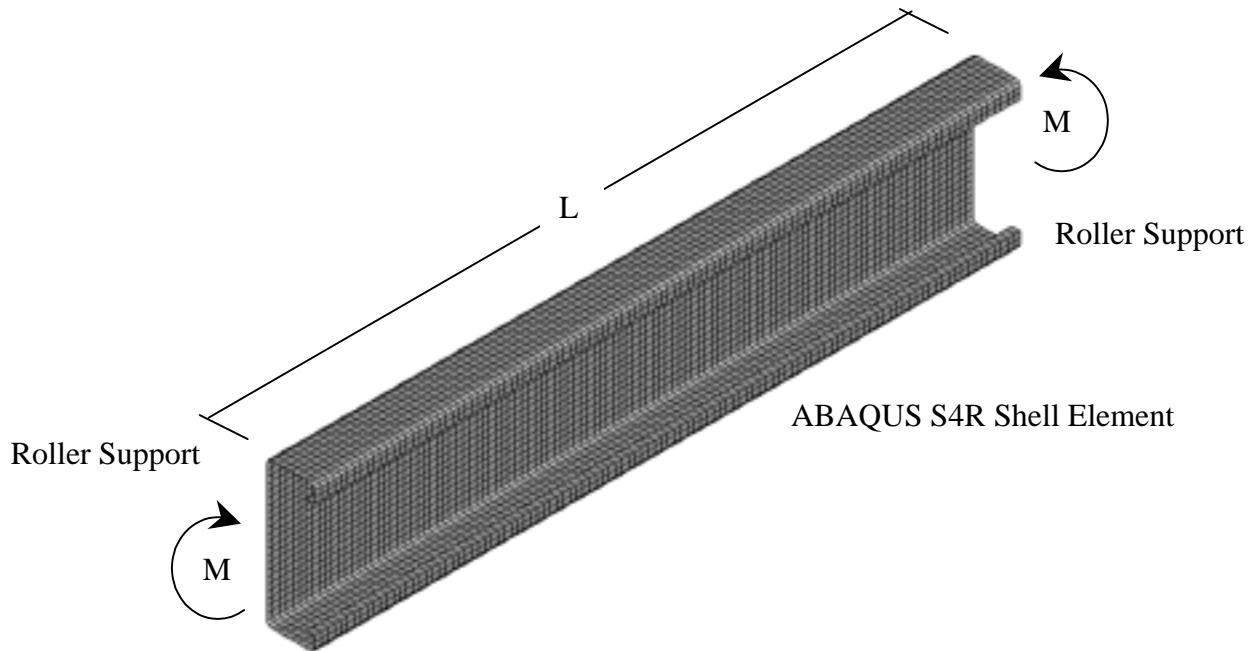


Figure 6.2 Boundary and Loading Condition

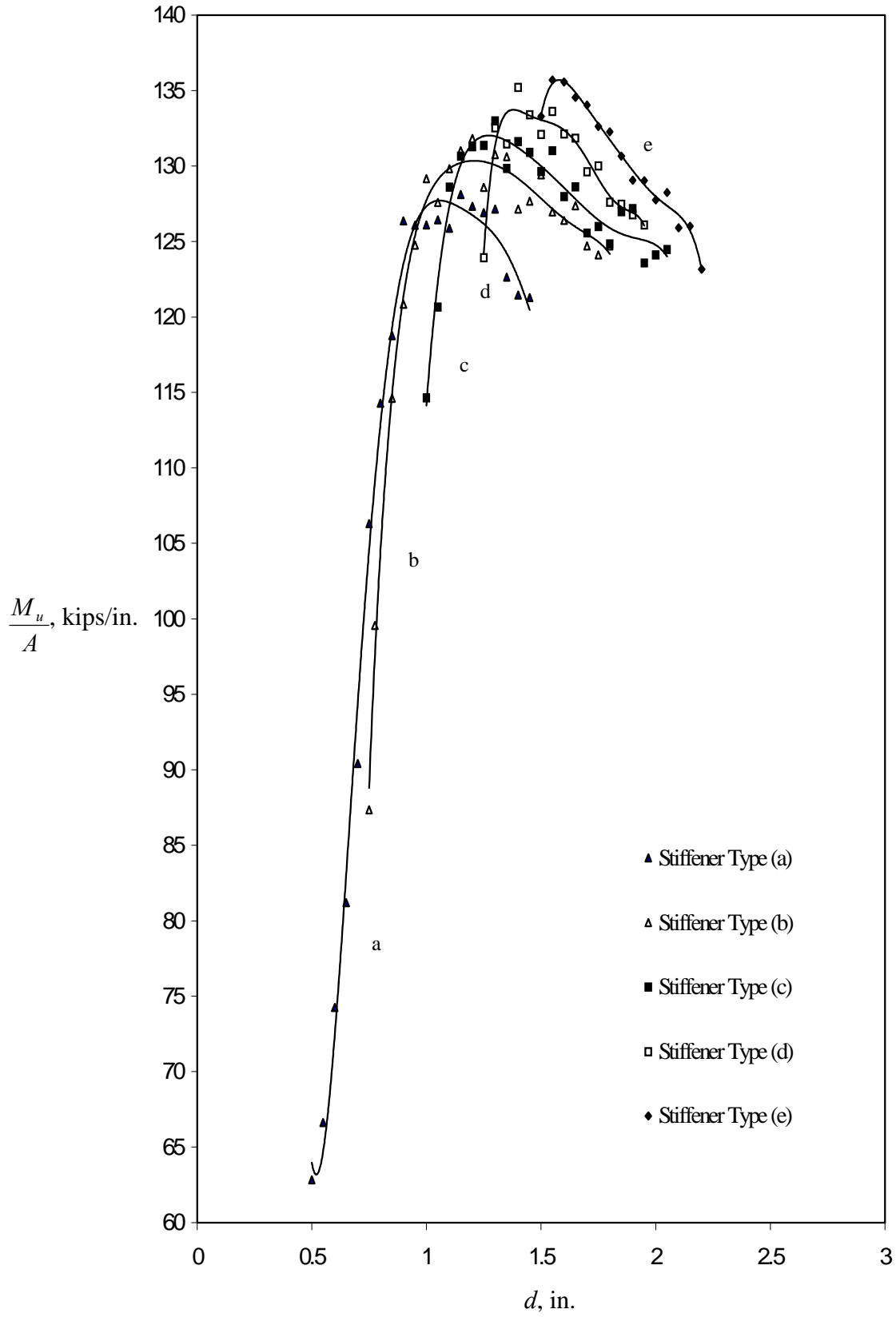


Figure 6.3 Inside Angled Optimization by Finite Element Method

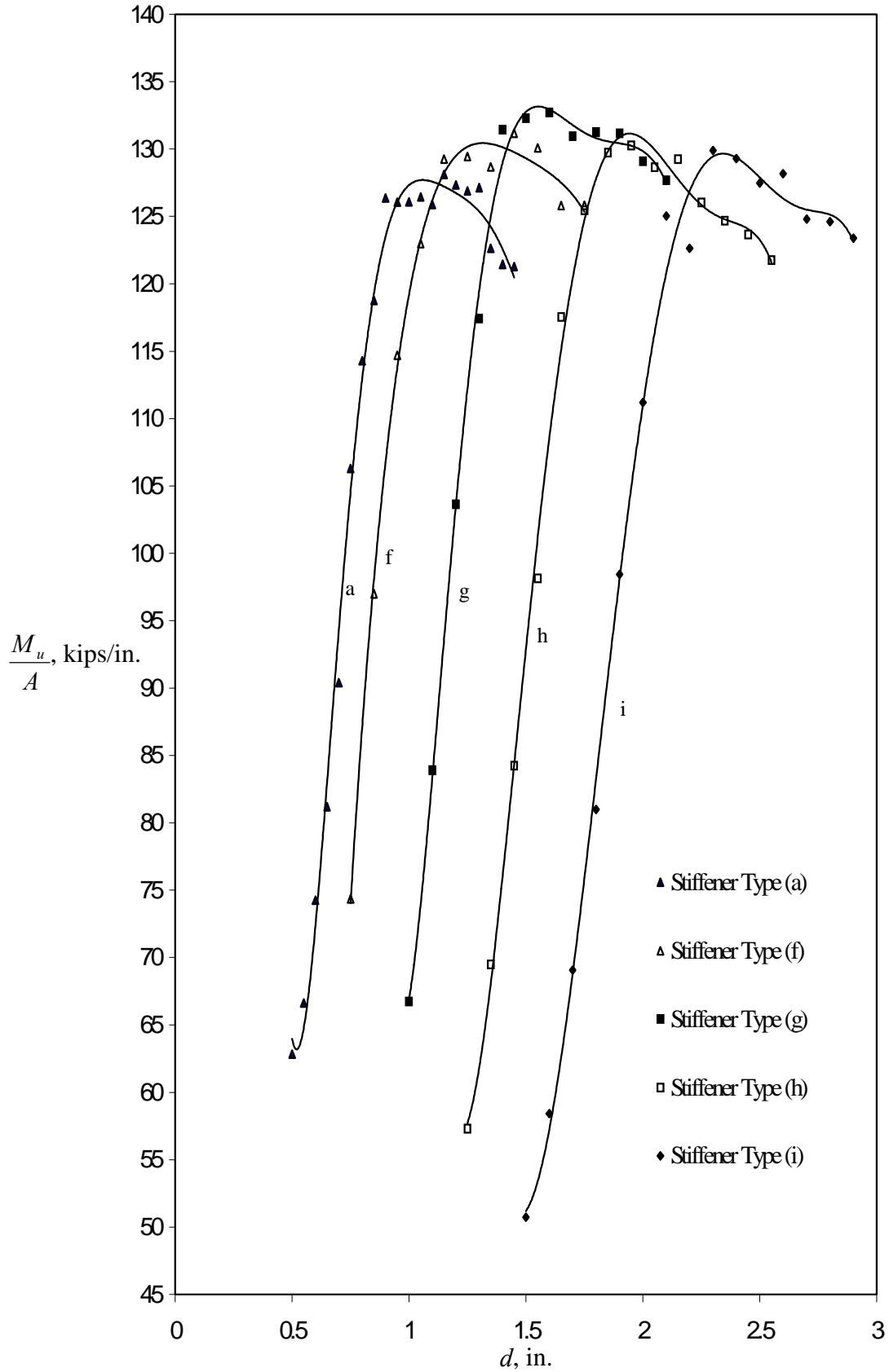


Figure 6.4 Outside Angled Optimization by Finite Element Method

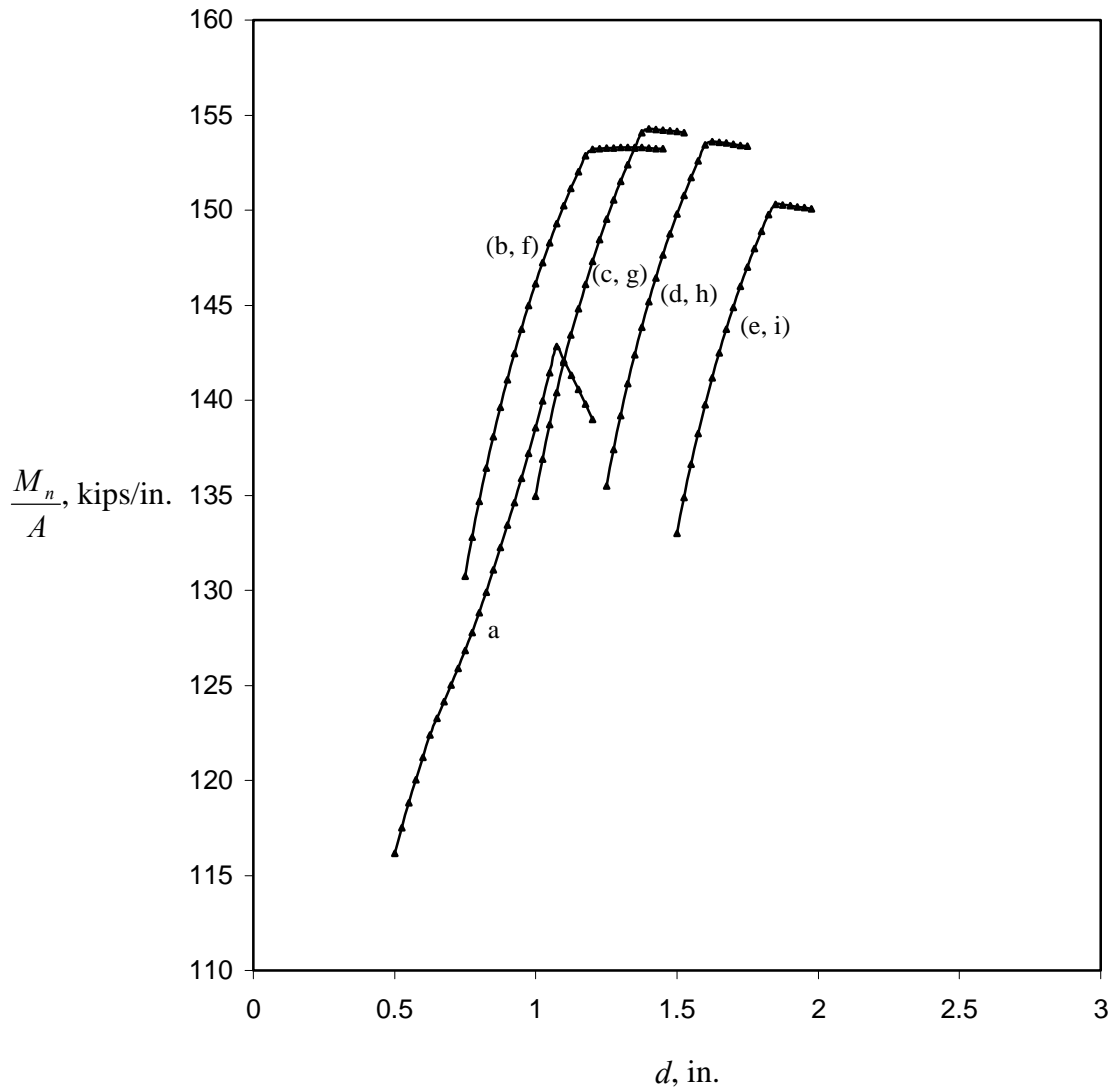


Figure 6.5 Inside and Outside Angled Optimization by AISI Method

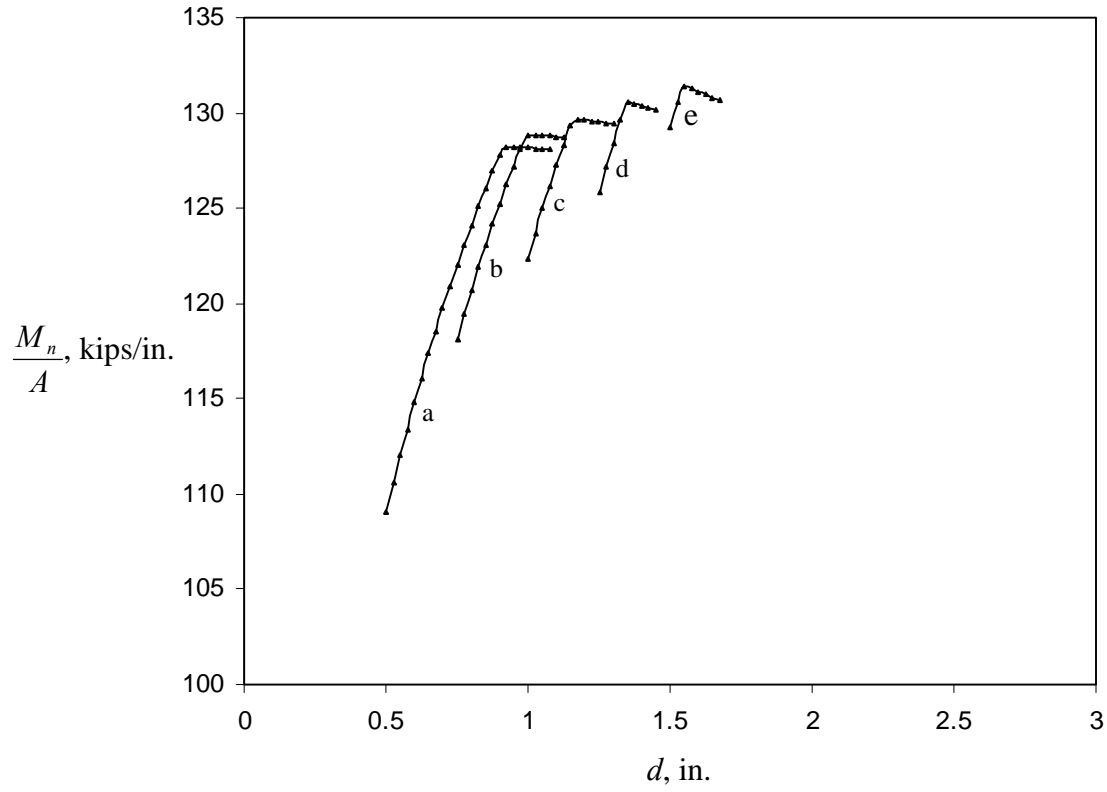


Figure 6.6 Inside Angled Optimization by Proposed Method

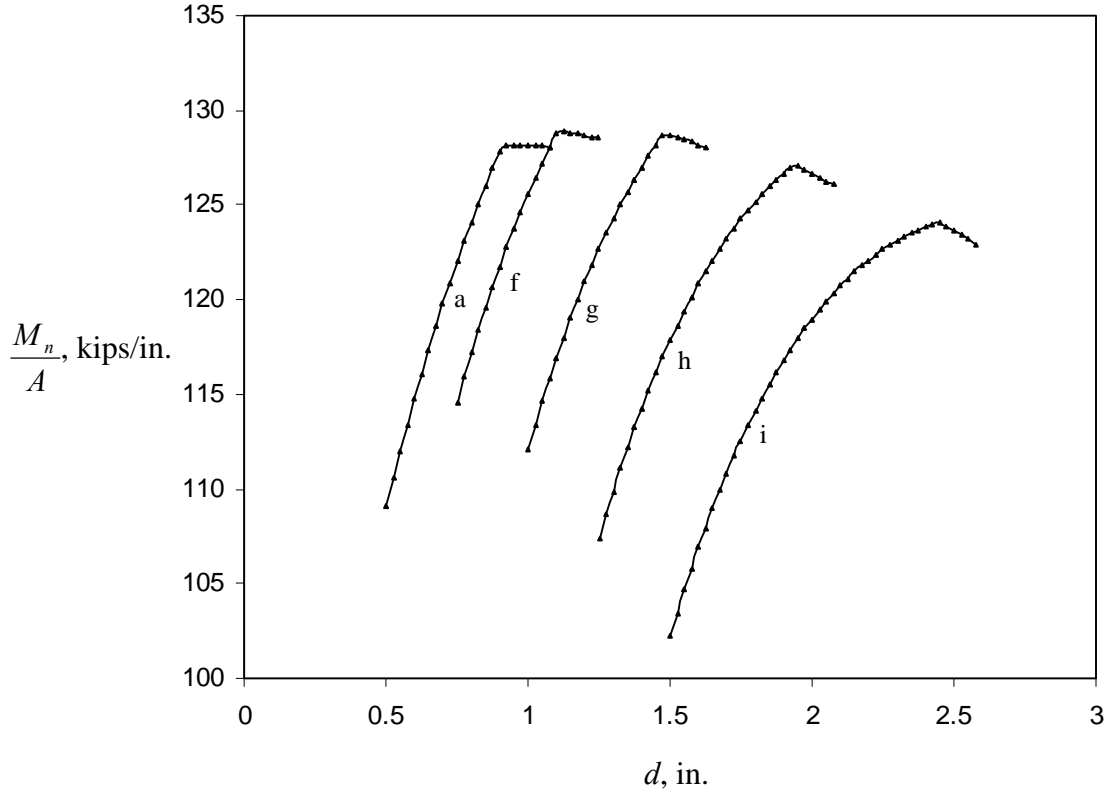


Figure 6.7 Outside Angled Optimization by Proposed Method

## **7 An Experimental Study**

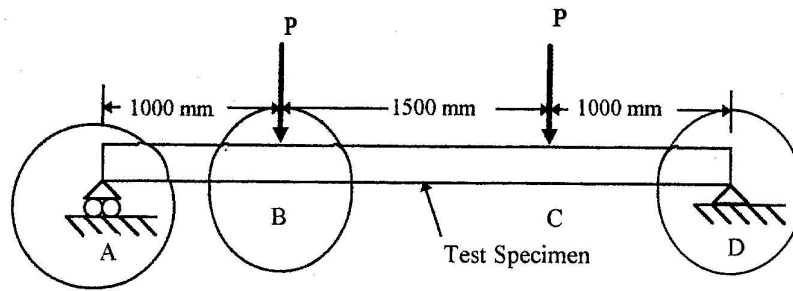
The two-point bending test shown in Figure 7.1 performed by Rinchen (1998) is studied here. A full finite element simulation of the experimental arrangement is carried out to compare with the physical test results.

### **7.1 Finite Element Simulation of Experimental Arrangement**

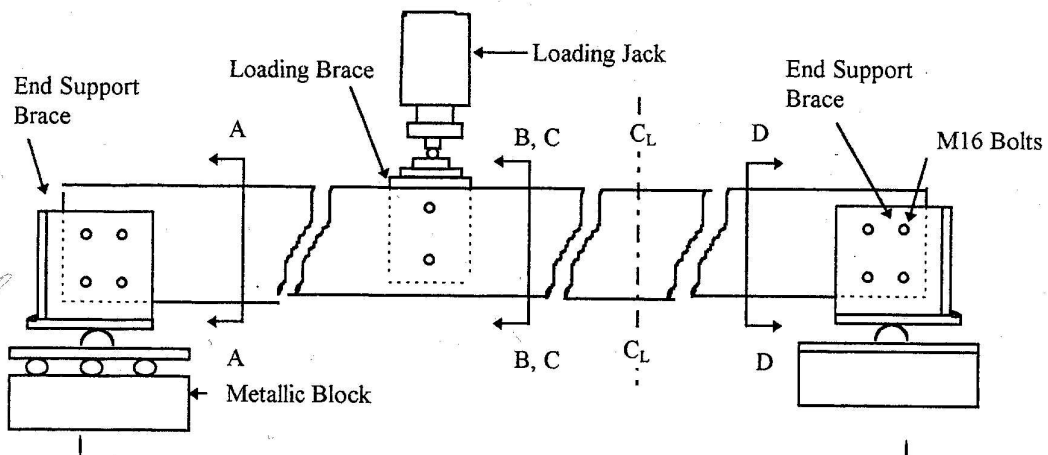
The two-point bending experiment setup shown in Figure 7.1 is simulated by FEM. A pair of DHS200-1.8 sections is placed back-to-back with loading braces and end support plates, which were modeled with a much thicker shell element compared to the ones used for the DHS200-1.8 members and connected only at the bolts positions. Pins and rollers were used at the end supports and concentrated loads. Residual stresses are also introduced as in the previous study but no imperfections are incorporated in to the analysis due to lack of actual imperfection measurement at the preliminary studies. However, a first approximation can be obtained and results are expected to be reduced when there are some imperfections. Figure 7.2 and 7.3 shows the model of this simulation at the failure state.

### **7.2 Conclusion**

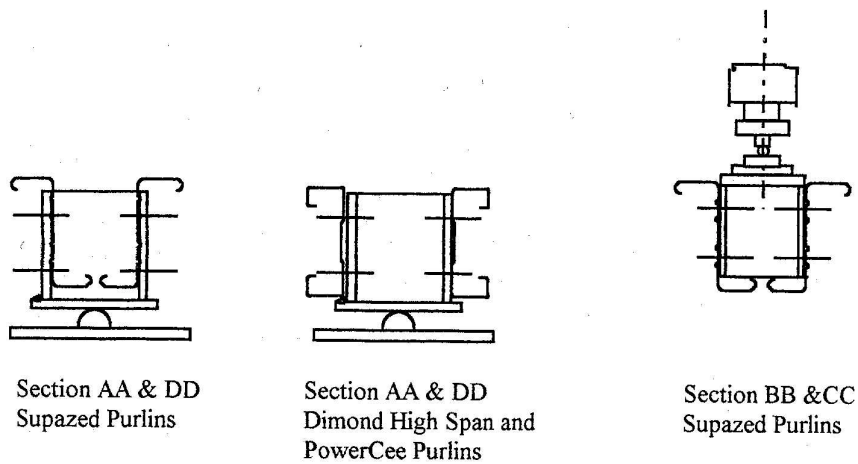
As suggested by Rinchen (1998), due to the interaction of local buckling, distortional buckling and shear at the loading points, tests resulted in providing section strength for localized failure instead of providing the section strength for pure bending. FEM simulation of the experiment arrangement with no imperfections incorporated showed even lower moment capacity with the same local failure mode. Results are shown and compared in Figure 7.4. Modification in the experimental arrangement is needed and should be simulated by finite element before doing actual testing. Rinchen (1998) also suggested some changes in the experimental set up.



(a) General Arrangement



(b) Details of Experimental Arrangement in Elevation



(c) Details of End Brace and Loading Brace

Figure 7.1 Details of Arrangement of Experiment, Rinchen (1998)



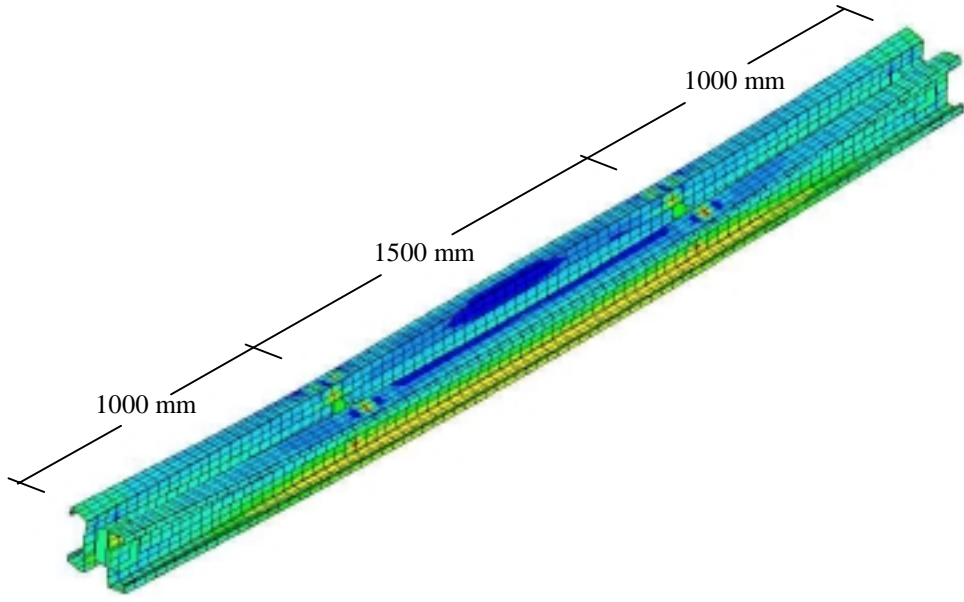


Figure 7.2 Simulation of Experimental Arrangement

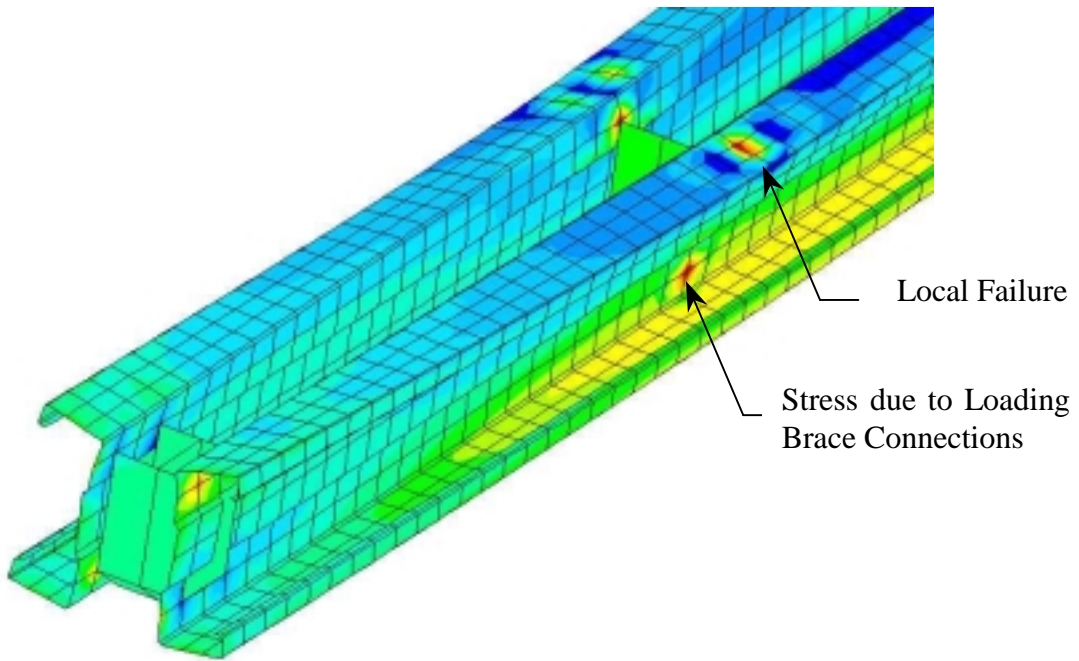


Figure 7.3 Failure Mode of DHS200-1.8 with Experimental Arrangement

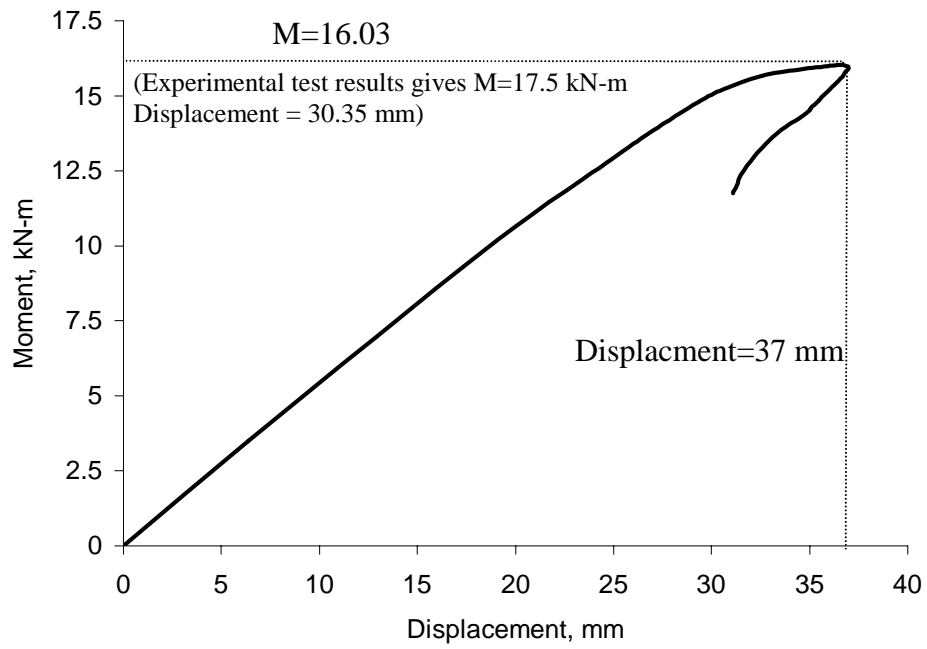


Figure 7.4 Finite Element Results for DHS200-1.8 with Experiment Arrangement

## 8 Initial Geometric Imperfection by Stochastic Process

The following is a joint study with Yongwook Kim in a structural reliability class. One of the most important factors that affect on the compressive strength in thin-walled sections such as cold-form steel is the geometric imperfection. In the previous study initial geometric imperfection was introduced by superimposing the eigenmodes for the local and distortional buckling. The magnitude of the imperfection was selected based on the statistical summary provided in Schafer (1997), Schafer and Peköz (1999). An alternative approach is studied here by using stochastic process to randomly generate signals for the imperfection geometric shape. From large numbers of the simulations, the relationships between ultimate compressive strength and standard deviation of the imperfection signal can be obtained. But because of lack of imperfection measurements there fore some assumptions had to be made first. The idea was to go threw the process and understand the approach.

### 8.1 Definitions and Assumptions

#### 8.1.1 Definitions of the Section

Rather than using a full cross section an isolated flange-stiffener model in uniform compression is studied instead and a nonlinear FEM analyses are performed using ABAQUS. To study only the simple lip stiffened element an idealization of the boundary condition at the web/flange junction is made by restraining all degrees of freedom except for the translation along the length. Roller supports are used at both ends and to avoid localized failure at the ends the uniform load has been distributed to the first row of elements. The Geometry and boundary conditions are shown in Figure 8.1. The material model used is elastic-plastic with strain hardening and  $f_y = 55$  ksi. Residual stress is also included with a 30% yield stress throughout the thickness in the longitudinal direction. The residual stresses are assumed tension on the outside and compression in the inside of the section. The length of the model is selected by using the length that would give the least buckling strength in the distortional mode, which is obtained, by using the Finite Strip Method CUFSM.

### 8.1.2 Definitions of the Imperfection

Only one type of imperfection shown in Figure 8.1 (b) is considered. The web to flange junction is assumed to be perfectly straight while the web to stiffeners junction will have longitudinal imperfection. The longitudinal imperfection signal is assumed to be zero mean, real valued stationary Gaussian stochastic process for a certain standard deviation because the imperfection is deviation from the perfect plane. Imperfection between the web junction and stiffeners junction is assumed to be linear.

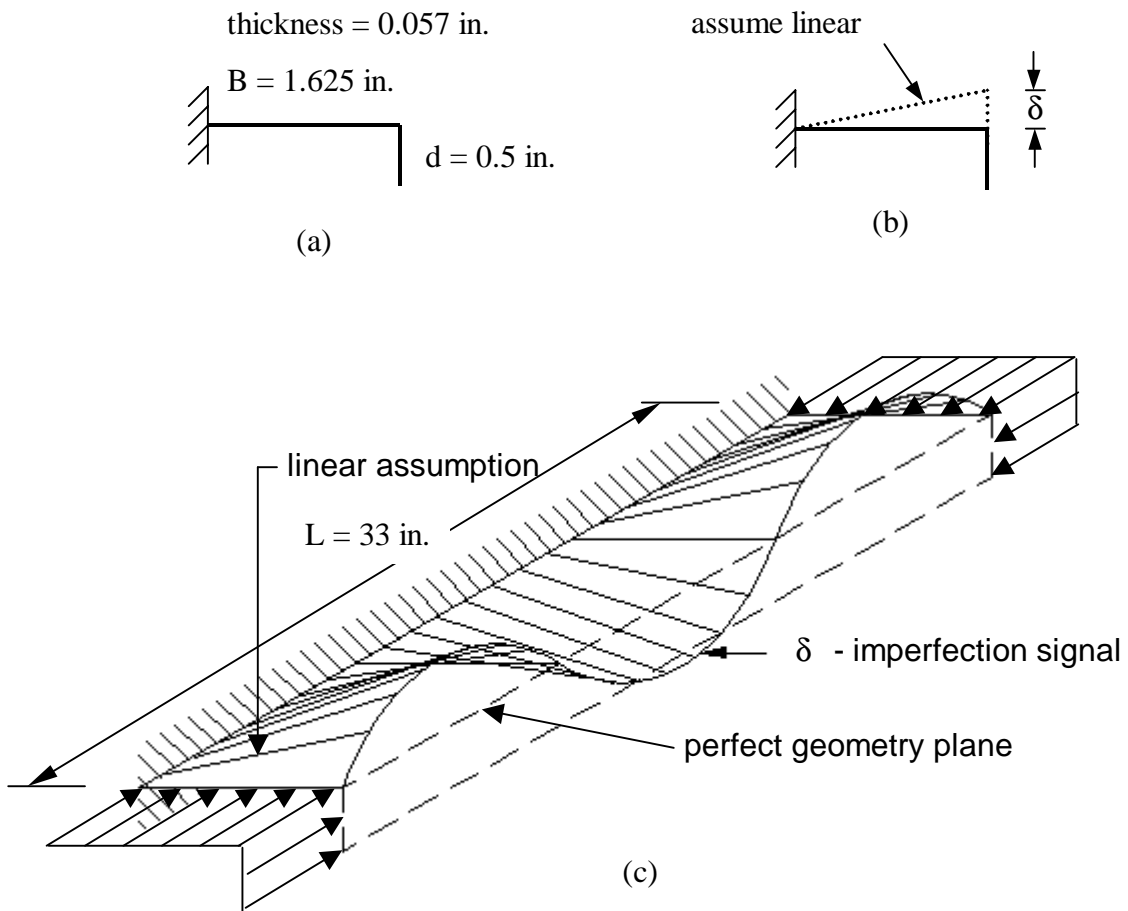


Figure 8.1 (a) Cross-section Geometry (b) Geometric Imperfection  
(c) Boundary Condition and Geometric Imperfection

## 8.2 Probabilistic Model for Uncertain Parameters

### 8.2.1 Imperfection is zero mean stationary Gaussian stochastic process:

A stochastic process is a function of two variables the parameter  $t$  and the probability parameter  $\omega$ .  $X(t, \omega)$ ,  $t \in T$ ,  $\omega \in \Omega$  where parameter  $t$  usually refers to time but for geometric imperfection  $t$  will denote as location along the length of the member. If a manufacturer makes the members in a same process and condition, a set of imperfection data  $X(\bullet, \omega)$  of the members can be called as sample, sample path or realization. In addition,  $X(\bullet, \omega)$  can also be called as a stationary stochastic process, since due to uniformity of the manufacturing process, the distributions of  $X$ 's over the different members ( $\omega$ ) for a fixed location 't' should be the identical regardless of its location ( $t$ ). This is because if  $X(t)$  is finite dimensional distribution and it does not change at a time shift for  $\forall n, \forall t_1, \dots, \forall t_n$ ,

$$F(x_1, x_2, \dots, x_n; t_1, t_2, \dots, t_n) = F(x_1, x_2, \dots, x_n; t_1', t_2', \dots, t_n')$$

then  $X(t)$  can be said to be stationary in the strict sense. That is, the distribution is only a function of time lag ( $t_k - t_k'$ ;  $k=1 \dots n$ )

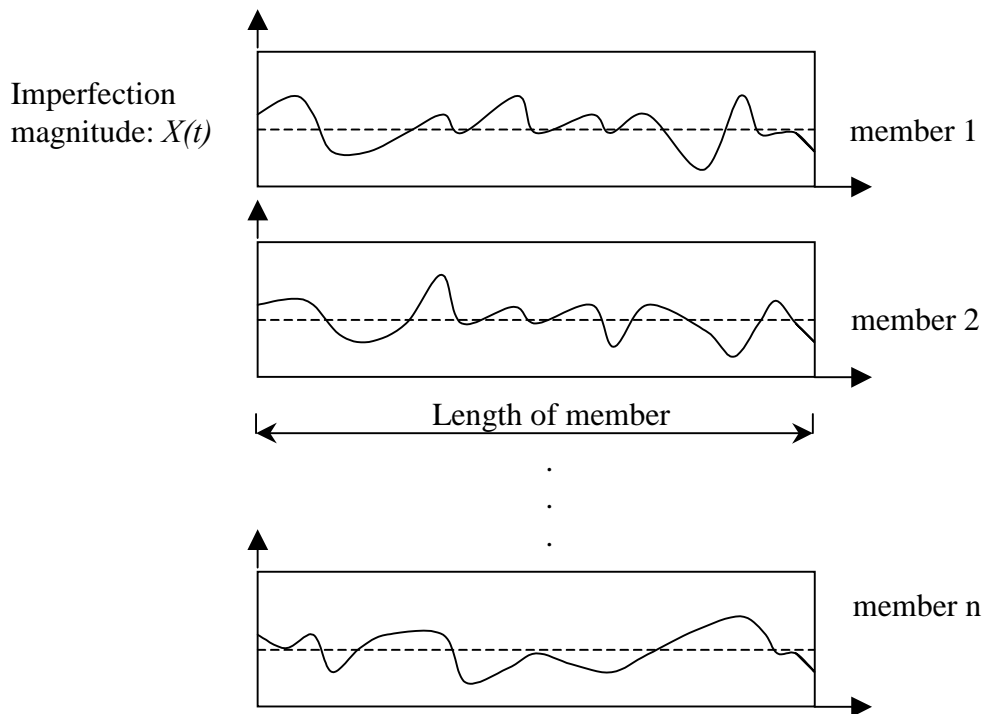


Figure 8.2 Imperfection Signal along the Length of the Member

### 8.2.2 Imperfection signal is assumed as:

$$\tilde{X}(t) = X_m(t) = \sum_{k=1}^m \sigma_k (V_k \cos(\omega_k t) + W_k \sin(\omega_k t))$$

If this process is zero mean, real valued, continuous, stationary Gaussian process then

$$X(t) = \int_0^{\infty} [\cos(\omega t) dU(\omega) + \sin(\omega t) dV(\omega)] \quad (*)$$

where  $U(\omega)$  and  $V(\omega)$  are real valued, zero mean, independent Gaussian process with properties  $E\{dU^2(\omega)\} = E\{dV^2(\omega)\} = G(\omega)d\omega$ . However, the measured imperfection data cannot be continuous, the equation (\*) cannot be obtained. Instead, discrete approximation of this process is possible. If this process is assumed to be a discrete version  $X(t)$  with one-sided truncated power. Then the stochastic process  $X(t)$  can be approximated as

$$\tilde{X}(t) = X_m(t) = \sum_{k=1}^m \sigma_k (V_k \cos(\omega_k t) + W_k \sin(\omega_k t))$$

where  $V_k$  and  $W_k$  are independent Gaussian random variables with zero means and unit variances.

$$\text{spectral density, } \tilde{G}(\omega) = \begin{cases} G(\omega), & 0 \leq \omega \leq \tilde{\omega} \\ 0, & \omega > \tilde{\omega} \end{cases}$$

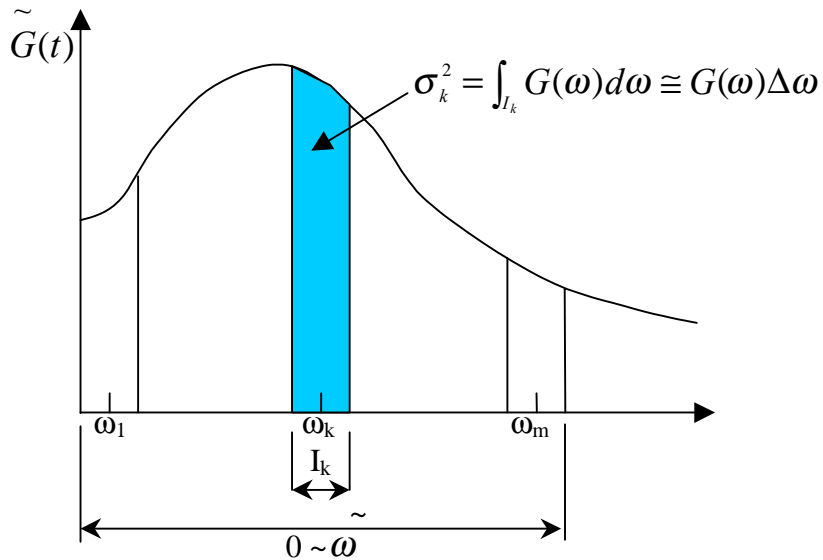


Figure 8.3 One-sided Power Spectral Density

### 8.2.3 Process is assumed as Band-Limited White Noise

If actual signals of imperfection from the measurement are available, the correlation function from the definition may be obtained. However, since the data is not available currently, the correlation function is assumed as Band-limited white noise, which is shown in Figure 8.4.  $R(\tau) = G_o \sin(\omega_c \tau) / \tau$ . where  $\omega_c = 1.6$  is selected from the experimental result of Schafer (1997) from the Fourier Transform of some cold-form steel members, truncation ( $\omega_c$ ) is decided at the 1.6 rad/in frequency. Standard deviation ( $\sigma_{imp,i}$ ) of imperfection parameter is constant and decided from the assumptions that it varies from 0.25 to 2.0 (0.25, 0.50, 0.75, 1.00, 1.25, 1.50, 1.75 and 2.00). These numbers are normalized standard deviation by dividing actual standard deviation by thickness 0.057 in. Then square of one of the selected standard deviation ( $\sigma_{imp,i}$ ) will be the total area of the one-sided spectral density. That is,  $(\sigma_{imp,i})^2 = G_{o,i} \times \omega_c$  where,  $i=1\sim 8$ . Then  $G_o$  is obtained for each different  $\sigma_{imp,i}$  and fixed  $\omega_c$ . In addition,  $\sigma_k^2$  can be computed from  $\sigma_k^2 = (\sigma_{imp,i})^2/5$ . Finally,  $V_k, W_k$  will be randomly generated.

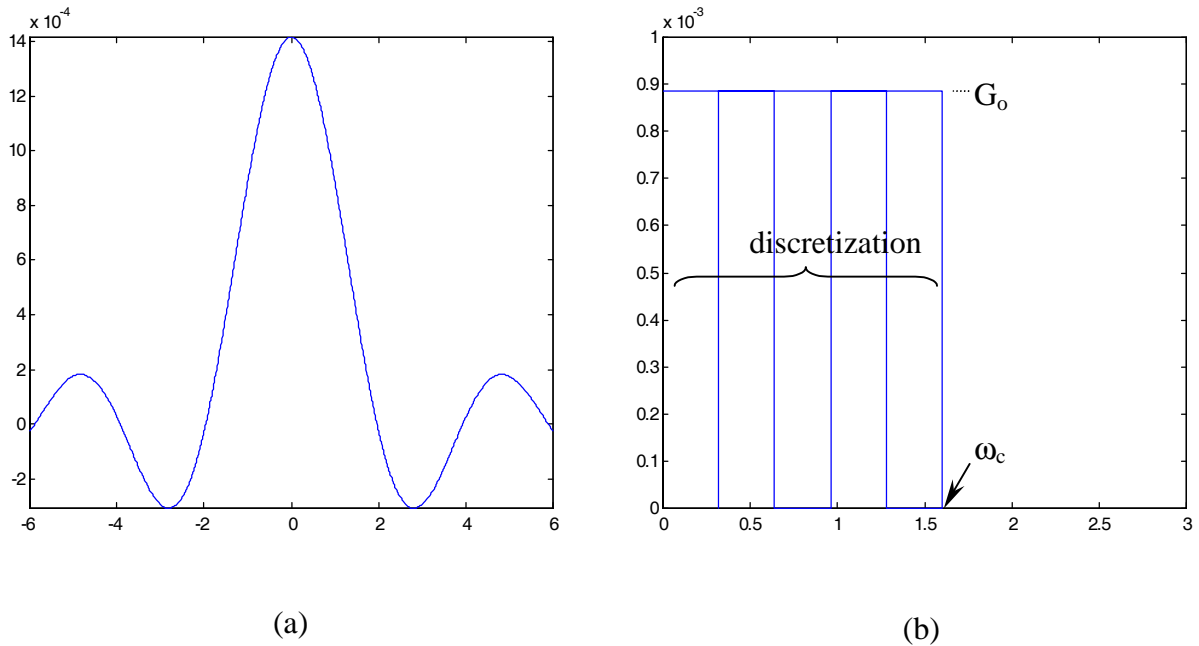


Figure 8.4 Band-Limited White Noise: (a) Correlation Function  
(b) One-sided Spectral Density,  $G(\omega)$

### 8.2.4 Generation of Imperfection Signal

Total 30 numbers of signals were generated for each standard deviation. Figure 8.5 and 8.6 shows 1 and 10 numbers of signals generated when  $\sigma_i = 1.0$  (=0.057 in.). For each standard deviation 30 numbers of signals were generated. Then with these, total 240 FEM analyses are implemented. Since 30 numbers of signals from one standard deviation are completely different, the ultimate capacities show scatter. For each scatter, histogram is made and relationships between ultimate compressive strength and standard deviation of the imperfection signal can be obtained. Results are shown in Figure 8.7 and 8.8.

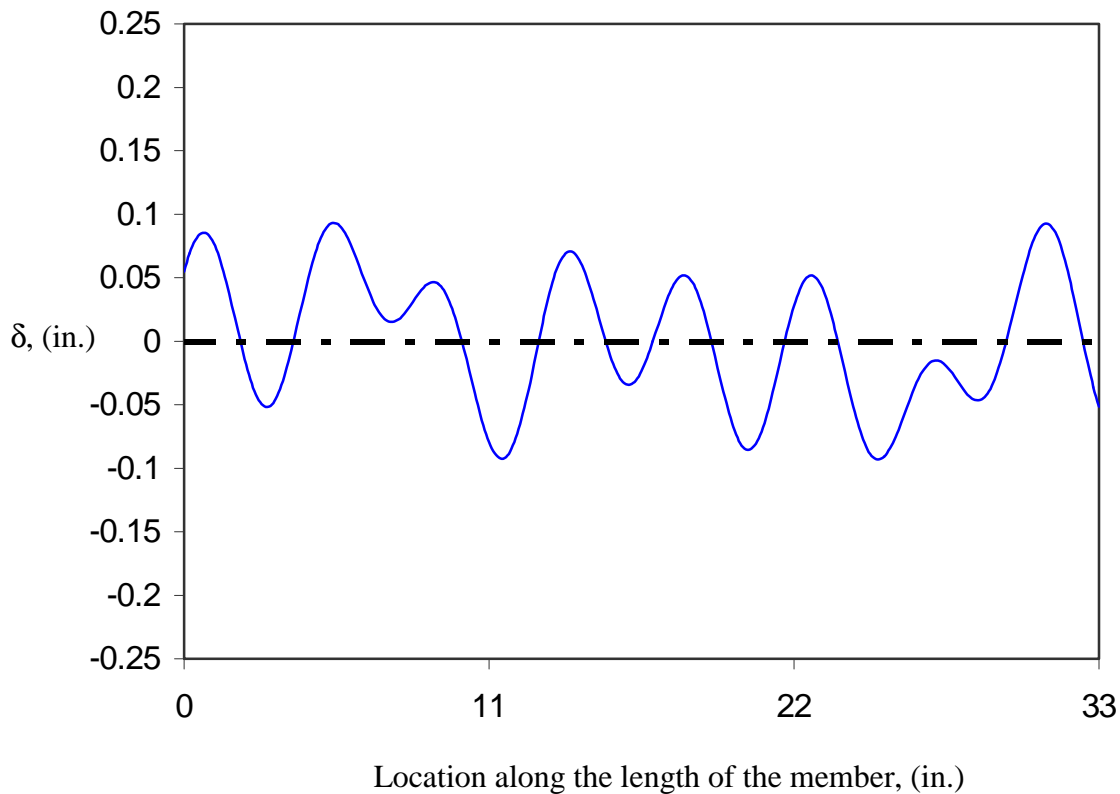


Figure 8.5 Zero mean,  $\sigma_i = 1.0$  Stationary Gaussian Stochastic Process: 1 Signal



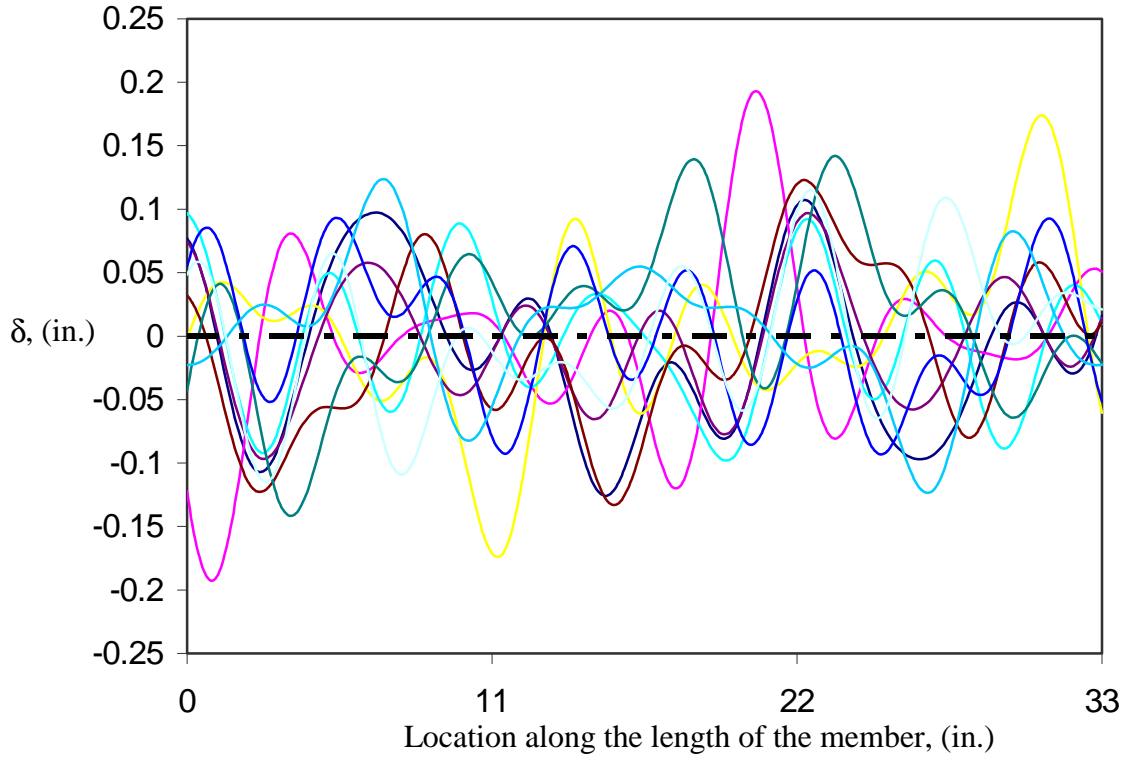


Figure 8.6 Zero mean,  $\sigma_i = 1.0$  Stationary Gaussian Stochastic Process: 10 Signal

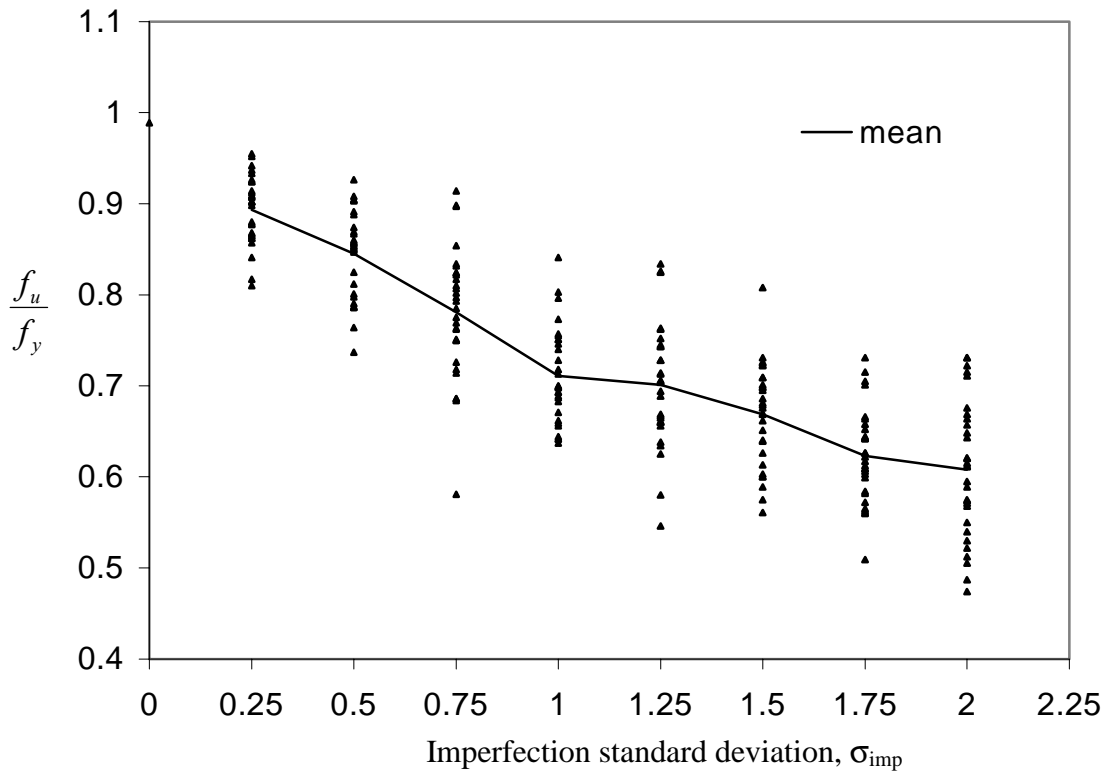


Figure 8.7 Ultimate Compressive Strength vs. Standard Deviation of the Imperfection Signal

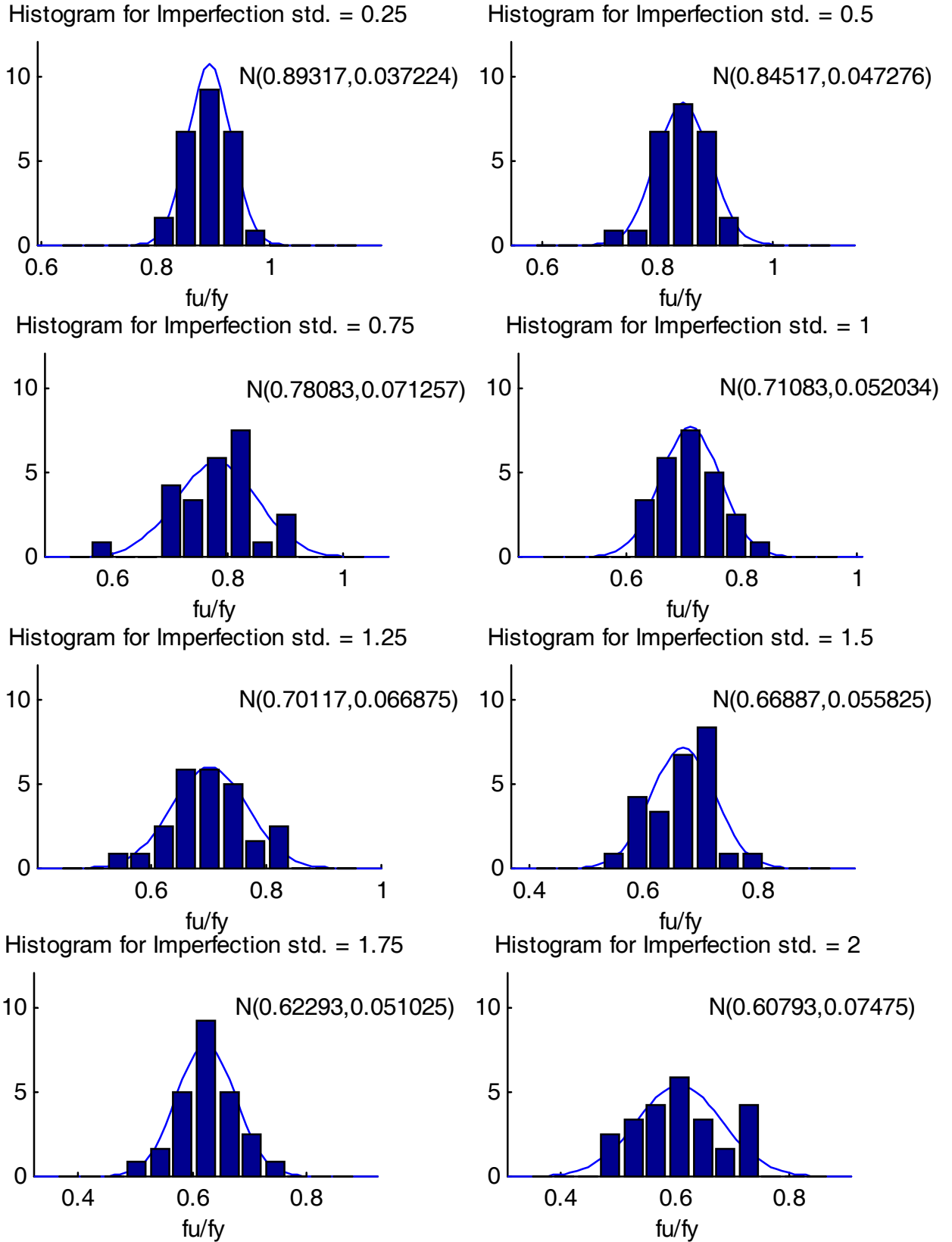


Figure 8.8 Histograms and Normal Distributions Density Function for each  $\sigma_{imp,i}$

### 8.3 Initial Geometric Imperfection by Eigenmodes

The same model as in Figure 8.1 is used again to compare the results of the two approaches initial geometric imperfection by eigenmodes and by stochastic process. The initial geometric imperfection is introduced by superimposing the eigenmodes for the local and distortional buckling in different combinations of their magnitudes. The imperfection shapes are shown in Figure 8.9. Magnitudes of the imperfection are selected equal to the imperfections used in the stochastic process. FEM results are shown in Table 8.1 and Figure 8.10.

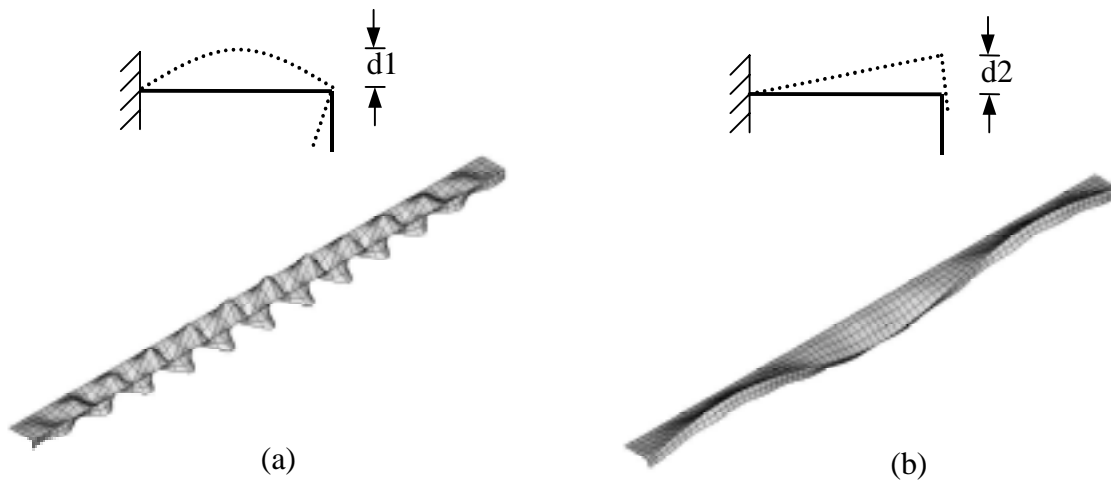


Figure 8.9 Eigenmodes (a) Local Buckling Imperfection Type 1,  $d1$   
(b) Distortional Buckling Imperfection Type 2,  $d2$

Table 8.1 FEM Results Ultimate Compressive Strength vs. Imperfection Type 1 vs. Type 2

		$d2, f_u/f_y$				
		0	0.5t	t	1.5t	2t
$d1, f_u/f_y$	0	0.989	0.812	0.679	0.624	0.581
	0.5t	0.967	0.811	0.693	0.625	0.58
	t	0.921	0.793	0.685	0.623	0.574
	1.5t	0.881	0.773	0.681	0.621	0.578
	2t	0.846	0.766	0.674	0.614	0.575

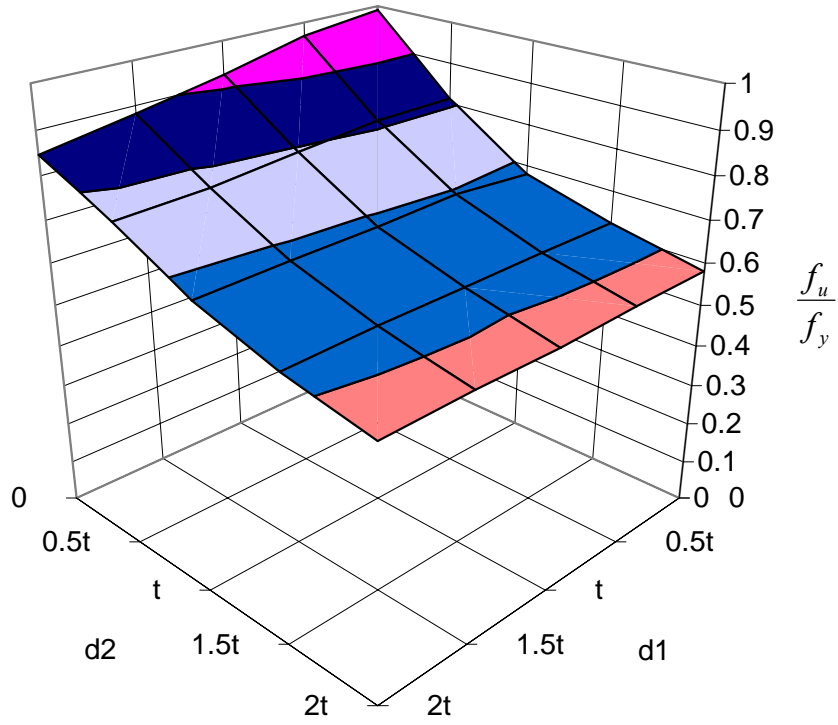


Figure 8.10 Ultimate Compressive Strength vs. Imperfection Type 1 vs. Type 2

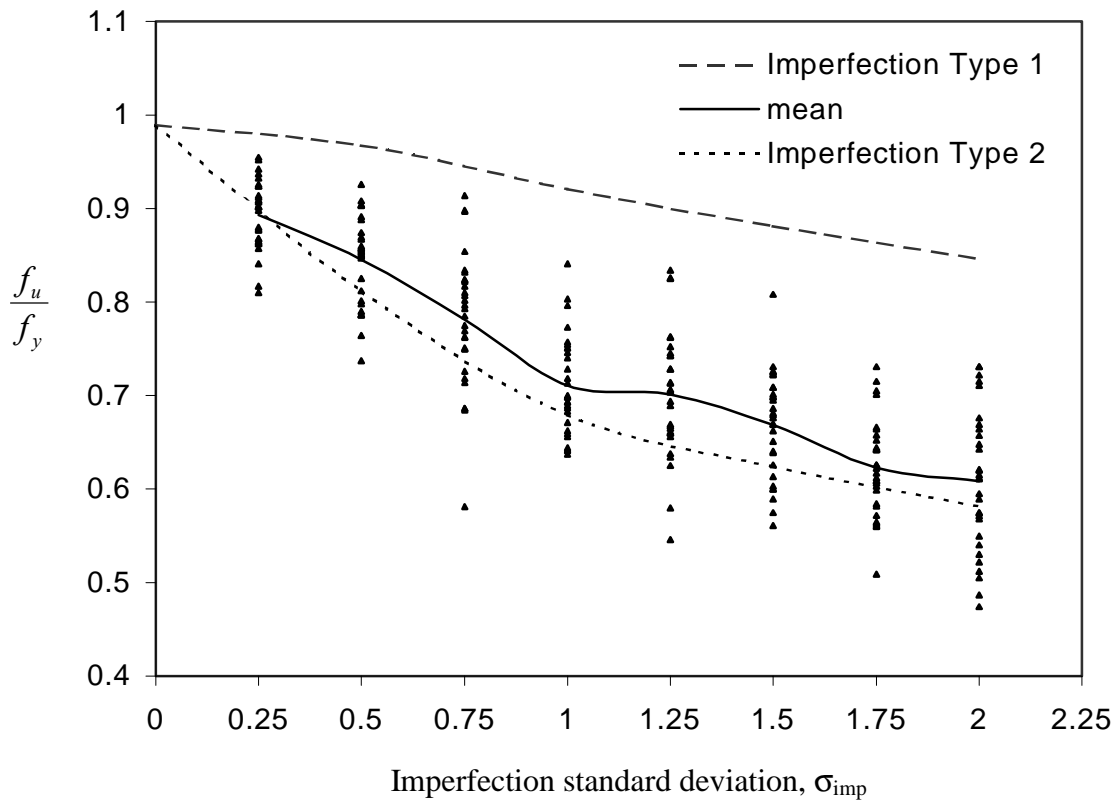


Figure 8.11 Initial Geometric Imperfection by Stochastic Process vs. Eigenmodes

## **8.4 Conclusion**

Results from both approaches initial geometric imperfection by stochastic process and by eigenmodes are compared in Figure 8.11. The imperfection type 1 and imperfection type 2 lines are the results from introducing only their own imperfection type which is the results from the first column and first row in Table 8.1 where there is no superimposing between the two modes. Results from the stochastic process and imperfection type 2 line is surprisingly close and both have significant impact on the strength reduction then the imperfection type 1. This shows that the section studied here is more sensitive to fail in the distortional buckling then local buckling. Although there were some assumptions made for the stochastic process but once some actual measurement of the section in study is available developing a better mathematical model is possible. The stochastic process used in this study is a 1 - dimensional stochastic process. Further study in using a 2 - dimensional stochastic process that could simulate a more realistic geometric imperfection plane is also of interest.

## **9 Summary and Conclusions**

The approach formulated by Schafer (1997) also discussed in Schafer and Peköz (1999) for simple lip stiffeners is also satisfactory for complex stiffeners. This conclusion is based on finite element method studies. Finite element modeling agrees well with the Australian test results but improvement in the experimental arrangement should be done to avoid localized failure. Results from the cross section optimization study shows that it is more efficient to have inside angled stiffener then simple lip or outside angled stiffener.

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